



# Onsite Wastewater Disposal Feasibility Study



January 6, 2023  
September 9, 2024 – Revision #1

Job No. 20-139

Kim Withrow, REHS  
Environmental Health Division  
Napa County Planning, Building and Environmental Services Department  
1195 Third Street, Suite 210  
Napa, CA 94559

Re: Onsite Wastewater Disposal Feasibility Study for the  
A&B Vineyards LLC Winery Use Permit Modification Application  
5215 Solano Avenue, Napa, California APN 034-190-040

Dear Ms. Withrow:

At the request of A&B Vineyards LLC we have evaluated the process and sanitary wastewater flows associated with the proposed Use Permit Modification. The winery is currently under construction, so wastewater systems have been designed and permitted but not yet installed. As part of our work we have also analyzed the capacity of the permitted process and sanitary wastewater systems that will serve the winery facility to determine if they are adequate to serve the proposed changes in use.

Existing development on the property includes approximately six acres of vineyards, two wells, access roads, winery buildings under construction and the related access and utility infrastructure typical of this type of agricultural and winery development.

The Use Permit Modification application under consideration proposes the following characteristics:

- Wine Production:
  - 45,000 gallons of wine per year
  - Crushing, fermenting, aging and bottling
- Employees:
  - 5 combined full-time and part time
- Marketing Plan:
  - Daily Tours and Tastings by Appointment
    - 34 visitors per day maximum

- Marketing Events Type #1
  - 12 per year
  - 30 guests maximum
  - Food prepared offsite by catering company
- Marketing Events Type #2
  - 2 per year
  - 125 guests maximum
  - Food prepared offsite by catering company
  - Portable toilets used for restrooms

Please see the A&B Vineyards LLC Use Permit Modification Conceptual Site Improvement Plans prepared by Applied Civil Engineering (attached) for approximate locations of existing and proposed facilities.

The remainder of this letter describes the process and sanitary wastewater disposal system design capacities, peak flows associated with the proposed changes in use and our analysis and recommendations related to the existing permitted but not installed process and sanitary wastewater disposal systems' ability to handle the anticipated wastewater flows.

#### Permitted Process Wastewater System

The permitted and constructed process wastewater treatment and disposal system consists of a Specialty Treatment Solutions membrane bioreactor pretreatment system followed by subsurface disposal via a geoflow drip type dispersal field. The system was designed for a calculated peak flow of 1,000 gpd however the pretreatment system was sized for 2,500 gpd instantaneous maximum to allow for operational flexibility and surge flows. The subsurface drip dispersal field was sized for 1,000 gpd which required 834 lineal feet (1,667 sf) of drip tubing however the field size was increased slightly to make best use of the available space and provide 1,200 lineal feet (2,400 sf) of drip tubing.

#### Permitted Sanitary Wastewater System

The permitted and constructed sanitary wastewater treatment and disposal system consists of an Orenco AdvanTex pretreatment system followed by subsurface disposal via a geoflow drip type dispersal field. The system was designed for a calculated peak flow of 255 gpd which required 213 lineal feet (425 sf) of drip tubing however the field size was increased slightly to make best use of the available space and provide 300 lineal feet (600 sf) of drip tubing.

#### Proposed Process Wastewater Design Flows

We have used the generally accepted standard that six gallons of winery process wastewater are generated for each gallon of wine that is produced each year and that 1.5 gallons of wastewater are generated during the crush period for each gallon of wine that is produced. Based on the 45,000-gallon production capacity and the expectation that both white and red wine will be produced at the winery, we have assumed a conservative 45 day crush period. Using these

assumptions, the annual, average daily and peak winery process wastewater flows are calculated as follows:

$$\text{Annual Winery Process Wastewater Flow} = \frac{45,000 \text{ gallons wine}}{\text{year}} \times \frac{6 \text{ gallons wastewater}}{1 \text{ gallon wine}}$$

$$\text{Annual Winery Process Wastewater Flow} = 270,000 \text{ gallons per year}$$

$$\text{Average Daily Process Wastewater Flow} = \frac{270,000 \text{ gallons wastewater}}{\text{year}} \times \frac{1 \text{ year}}{365 \text{ days}}$$

$$\text{Average Daily Winery Process Wastewater Flow} = 740 \text{ gallons per day}$$

$$\text{Peak Winery Process Wastewater Flow} = \frac{45,000 \text{ gallons wine}}{\text{year}} \times \frac{1.5 \text{ gallons wastewater}}{1 \text{ gallon wine}} \times \frac{1 \text{ year}}{45 \text{ crush days}}$$

$$\text{Peak Winery Process Wastewater Flow} = 1,500 \text{ gallons per day (gpd)}$$

#### Proposed Winery Sanitary Wastewater Design Flows

The peak sanitary wastewater flow from the winery is calculated based on the number of winery employees, the number of daily visitors for tastings and the number of guests attending scheduled marketing events. In accordance with Table 4 of the Napa County "Regulations for Design, Construction, and Installation of Alternative Sewage Treatment Systems" we have used a design flow rate of 15 gallons per day per employee and 3 gallons per day per visitor for tastings. Table 4 does not specifically address design wastewater flows for guests at marketing events. For marketing events that will have catered meals that are prepared offsite we have estimated 5 gallons of wastewater per guest assuming there will be no food preparation or cleanup onsite. All events with more than 30 guests will utilize portable toilets. Based on these assumptions, the peak winery sanitary wastewater flows are calculated as follows:

#### Employees

$$\text{Peak Sanitary Wastewater Flow} = 5 \text{ employees} \times 15 \text{ gpd per employee}$$

$$\text{Peak Sanitary Wastewater Flow} = 75 \text{ gpd}$$

#### Daily Tastings

$$\text{Peak Sanitary Wastewater Flow} = 34 \text{ visitors per day} \times 3 \text{ gallons per visitor}$$

$$\text{Peak Sanitary Wastewater Flow} = 102 \text{ gpd}$$

#### Marketing Events #1 (12 per year)

$$\text{Peak Sanitary Wastewater Flow} = 30 \text{ guests} \times 5 \text{ gallons per guest}$$

$$\text{Peak Sanitary Wastewater Flow} = 150 \text{ gpd}$$

#### Marketing Events #2 (2 per year)



Peak Sanitary Wastewater Flow = 125 guests X 5 gallons per guest  
Peak Sanitary Wastewater Flow = 625 gpd

#### Total Peak Winery Sanitary Wastewater Flow

In order to manage the peak sanitary wastewater flows to the disposal field portable toilets will be used for all events with more than 30 guests in attendance and daily tours and tastings will not occur on marketing event days. Therefore, the worst-case peak winery sanitary wastewater flow is calculated based on 5 employees and a marketing event for 30 people. The peak flow for this scenario is calculated as follows:

Total Peak Winery Sanitary Wastewater Flow = 75 gpd + 150 gpd

*Total Peak Winery Sanitary Wastewater Flow = 225 gpd*

### **Existing Process and Sanitary Wastewater System Capacities**

#### Process Wastewater System Capacity

As noted above the permitted process wastewater system overall design capacity is 1,000 and the pretreatment unit can handle peak day maximums of 2,500 gpd.

#### Sanitary Wastewater System Capacity

As noted above the permitted sanitary wastewater system design capacity is 255 gpd.

### **Proposed Design Flow vs Existing Capacity**

#### Process Wastewater System Capacity

The predicted Peak Winery Process Wastewater Flow for the proposed winery operational characteristics (1,500 gpd) is more than the design capacity of the permitted subsurface drip dispersal field (1,000 gpd) but is less than the capacity of the pretreatment system (2,500 gpd). As noted above the design includes 2,400 square feet of dispersal field area which is well in excess of the required 1,667 square feet. This corresponds to an actual capacity of 1,440 gpd based on an application rate of 0.6 gpd/sf which is slightly less than the predicted Peak Winery Process Wastewater Flow (1,500 gpd).

#### Sanitary Wastewater System Capacity

The predicted Peak Winery Sanitary Wastewater Flow for the proposed winery operational characteristics (225 gpd) is less than the design capacity of the permitted subsurface drip dispersal field (255 gpd). As noted above the design includes 600 square feet of dispersal field area which is well in excess of the required 425 square feet. This corresponds to an actual capacity of 360 gpd based on an application rate of 0.6 gpd/sf which is also in excess of the predicted Peak Winery Sanitary Wastewater Flow (225 gpd).

## **Recommendations**

The predicted Peak Winery Process Wastewater Flow exceeds the capacity of the existing system but the predicted Peak Winery Sanitary Wastewater Flow is within the capacity of the existing system. Therefore, design adjustments are needed to accommodate the new process wastewater flow increase. Improvements are not needed for the sanitary wastewater system as the existing permitted design is adequate to handle the proposed flows.

We have explored options for modifying the design of the winery process wastewater system to accommodate the increased flows. Our recommendations below focus only on the process wastewater system.

### Option #1 – Expand Existing System

The 1,500 gpd design flow equates to 2,500 sf of subsurface drip dispersal area based on the 0.6 gpd/sf loading rate. The primary area could easily be expanded from the current design of 2,400 sf to the required 2,500 sf in the area of the previously tested soil. All application of treated winery process wastewater to a subsurface disposal system must comply with the requirements of the Statewide General Waste Discharge Requirements (WDRs) for Wineries in California and the currently in-progress NOI for coverage under this permit will have to be revised accordingly.

### Reserve Area

A commensurate increase in reserve area (200% of primary area) would also be required to provide 5,000 sf of reserve area. This reserve area can be accommodated in the vicinity of the originally designed reserve area.

### Option #2 – Capture Treated PW and Re-Use for Irrigation

In this scenario the process wastewater would be collected in an irrigation storage tank and then be used for vineyard and/or landscape irrigation rather than being disposed of in the in-ground system. No change is needed to the pretreatment system as it can adequately handle the proposed design flows and provide water of the quality needed for surface irrigation.

### Process Wastewater Disposal / Re-Use for Irrigation

We propose that disposal of the treated winery process wastewater be via irrigation of the onsite vineyard (and/or potentially landscaping as well). For the purpose of this study we have assumed that the winery process wastewater will be applied to approximately four acres of vineyard that is located to the west of the new winery building and outside of the well setbacks. This is a conservative assumption to simplify this analysis as more vineyard is available outside of the required setbacks and the treated water can also be used for landscape irrigation. The final irrigation area will be determined and incorporated into the final design with the installation permit application.

In order to accommodate differences in the timing of wastewater generation, irrigation demand, and limitations on wet weather application of treated wastewater a storage tank will be required. We have prepared a water balance calculation to size a tank that will temporarily store treated wastewater generated from the winery before it is applied to the vineyard. The water balance

calculations assume a monthly winery process wastewater generation rate and a monthly vineyard irrigation schedule based on our past experience with projects of this type. The water balance further assumes that during the summer the treated wastewater will be used to offset the irrigation needs of the vineyard and in the winter application of treated winery process wastewater will not occur to prevent runoff. The water balance calculations show that the proposed land application area is large enough to accept all the wastewater generated each month throughout the irrigation system and that a tank with a volume of at least 81,000 gallons is required to capture water from the non-irrigation season to use during the irrigation season. year without carry over (see attached). This tank will also be able to contain more than a weeks' worth of peak flow to allow flexibility in irrigation scheduling during the harvest period.

All land application of treated winery process wastewater must comply with the requirements of the Statewide General Waste Discharge Requirements (WDRs) for Wineries in California and the currently in-progress NOI for coverage under this permit will have to be revised accordingly.

### Summary

The calculations presented above illustrate that the wastewater flows associated with the proposed Use Permit Modification will exceed the capacity of the permitted process wastewater system but will not exceed the capacity of the permitted sanitary wastewater system. However, there are at least two options for how to handle the planned increased process wastewater flow rates which include adding to the existing system and capturing the treated water and re-using it for irrigation.

We trust that this provides the information you need to process the subject Use Permit Modification. Please feel free to contact us at (707) 320-4968 if you have any questions.

Sincerely,

Applied Civil Engineering Incorporated

By:

**Michael R. Muelrath**

Michael R. Muelrath RCE 67435  
Principal



Copy:

Steve Contursi, A&B Vineyards LLC (via email)  
Donna Oldford, Plans4Wine (via email)

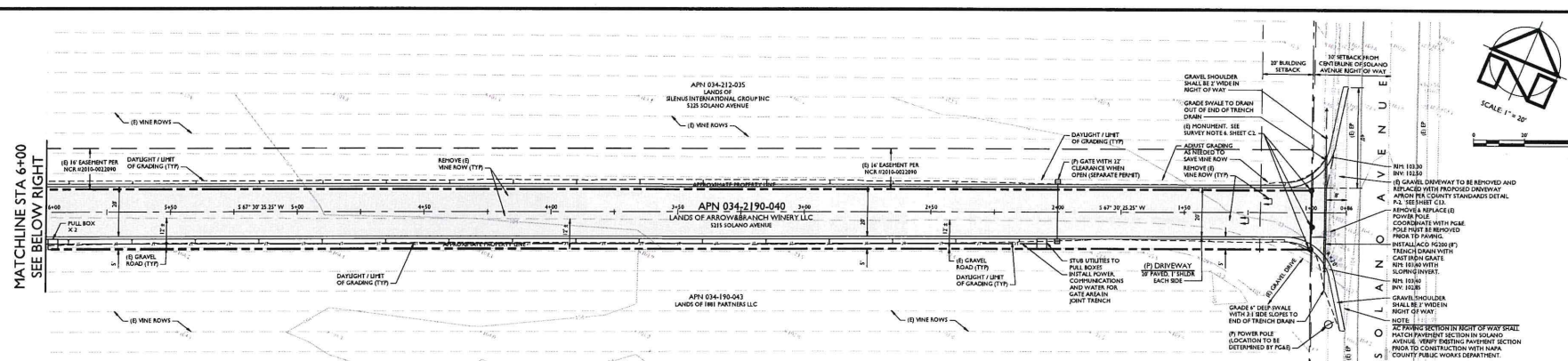
Attachments:

A&B Vineyards LLC Use Permit Modification Conceptual Site Improvement Plans  
Water Balance Spreadsheets

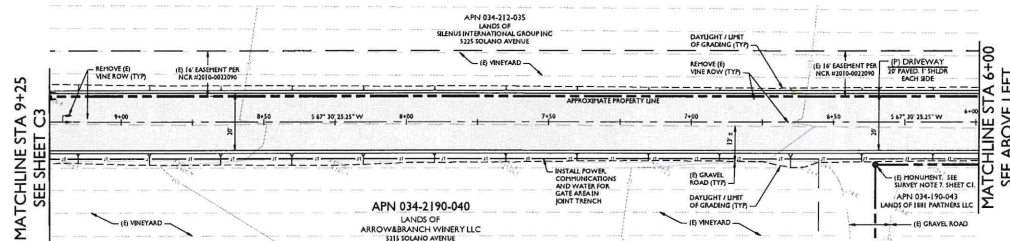




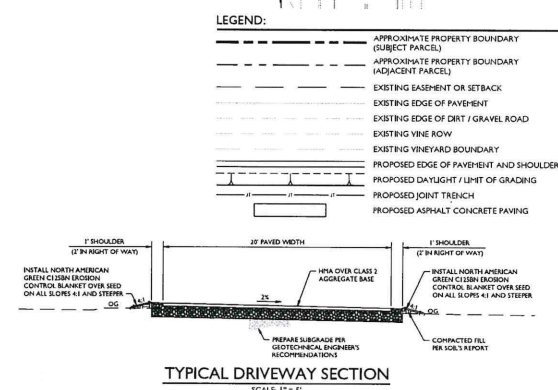
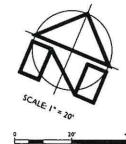




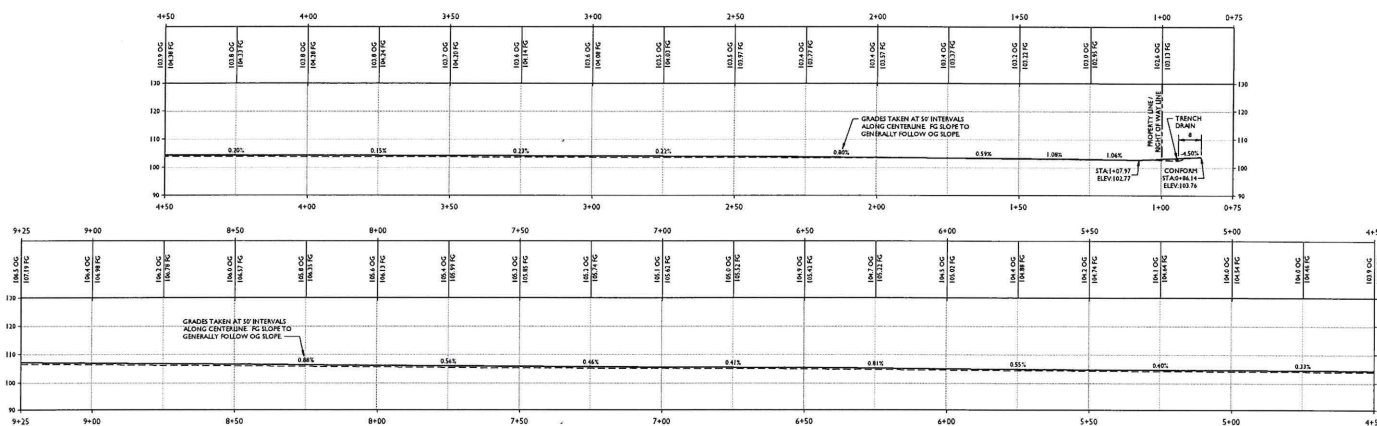
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SCALE: 1" = 20'



DRIVEWAY PLAN  
STA 6+00 TO STA 9+25  
SCALE: 1" = 20'



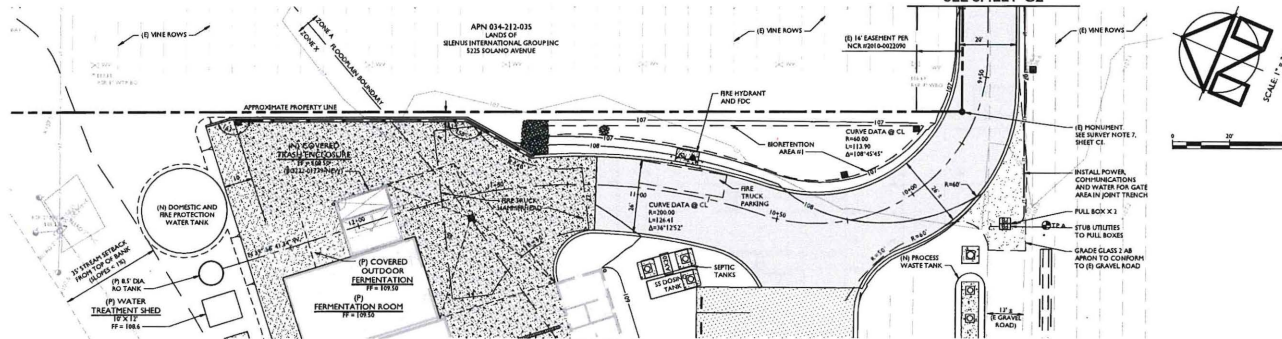
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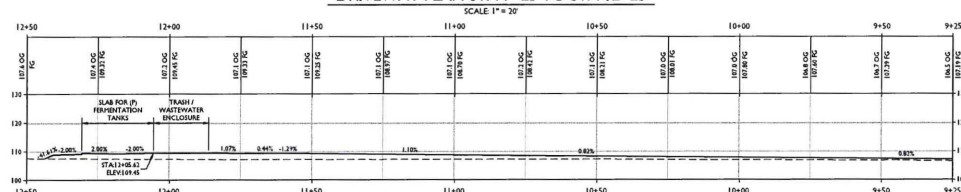
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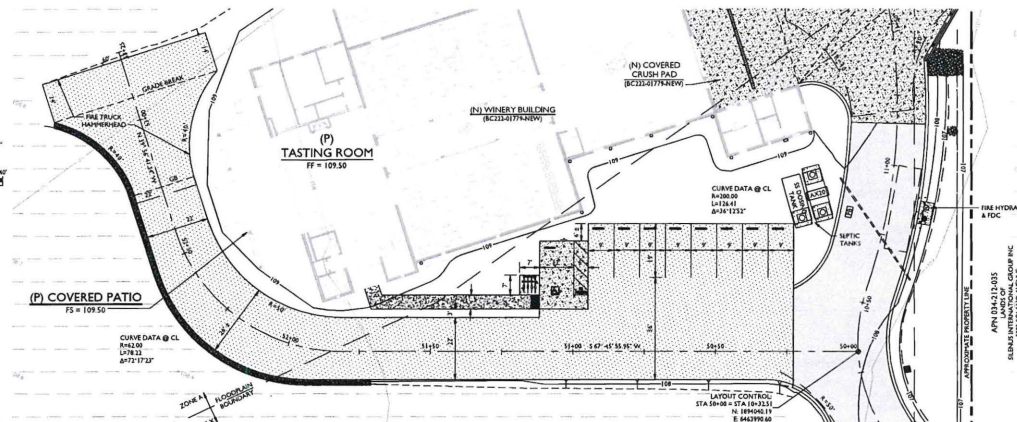
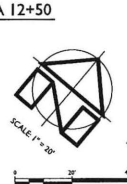
MATCHLINE STA 9+25  
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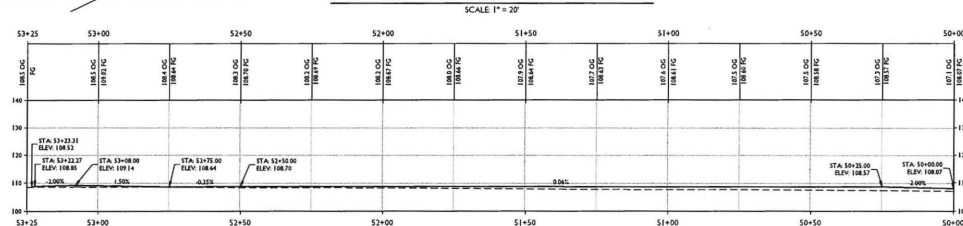
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DRIVEWAY PROFILE STA 9+25 TO STA 12+50

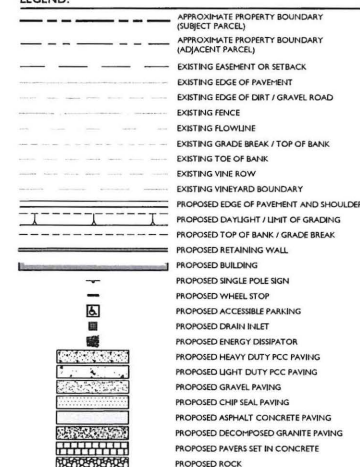


DRIVEWAY PLAN STA 50+00 TO STA 53+22



DRIVEWAY PROFILE STA 50+00 TO STA 53+25

LEGEND:



**APPLIED**

INCORPORATED  
22160 Jefferson Street, Suite 230  
Napá, CA 94559  
707/320-4968 | [www.appliedcivil.com](http://www.appliedcivil.com)

A&B VINEYARDS LLC

PERMIT MODIFICATION CONCEPTUAL SITE IMPROVEMENT PLANS  
DRIVEWAY PLAN & PROFILE STA 9+25 TO STA 53+25

PREPARED UNDER  
DIRECTION OF:



DRAWN BY:

CHECKED BY:

DATE: \_\_\_\_\_

REVISIONS:

PERMIT SUBMITT

5/10/2023  
PERMIT RESUBMIT

9/6/2023  
PERMIT RESUME

8/9/2024

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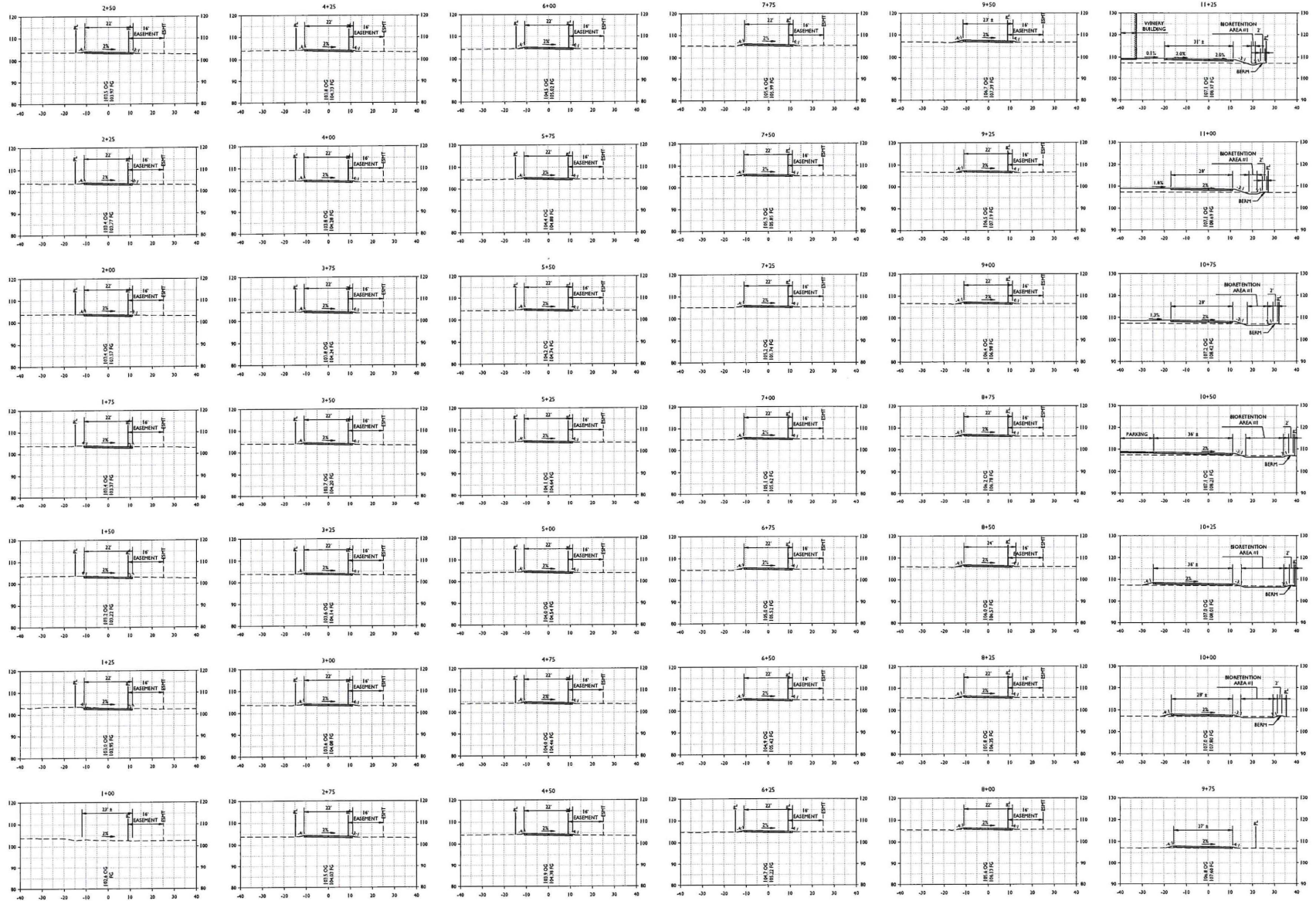
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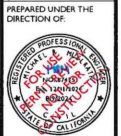
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DRIVEWAY CROSS SECTIONS STA 1+00 TO STA 11+25

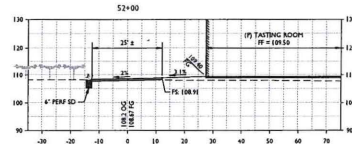
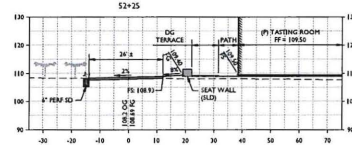
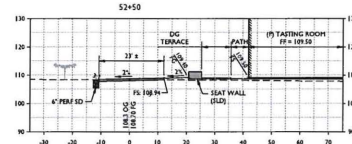
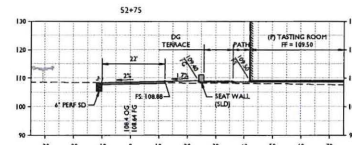
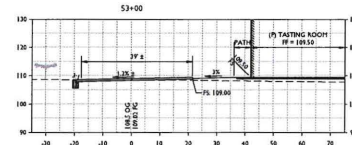
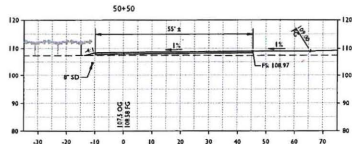
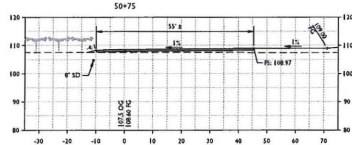
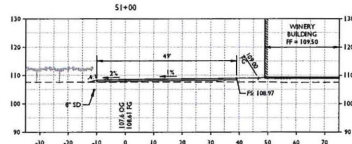
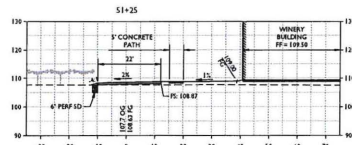
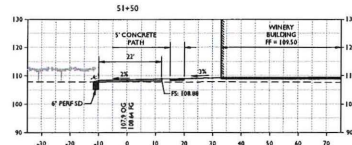
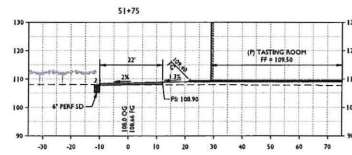
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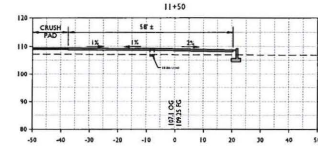
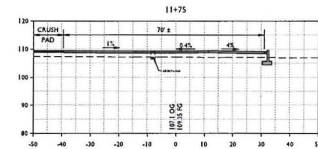
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| REVISIONS:  | BY                     |
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| 5/10/2023   | YHS PERMIT RESUBMITTAL |
| 7/6/2023    | YHS PERMIT RESUBMITTAL |
| 8/7/2024    | YHS PERMIT RESUBMITTAL |

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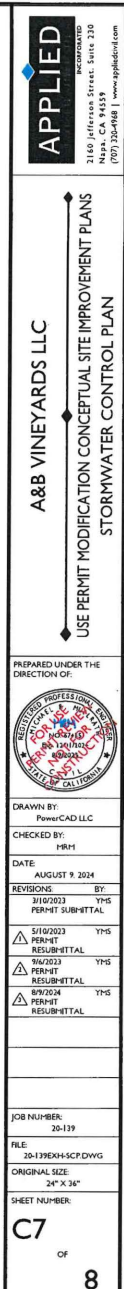
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| REVISIONS:  | BY: YHS<br>DATE: 5/10/2023<br>PERMIT SUBMITTAL |
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|             | DATE: 9/6/2023<br>YHS<br>PERMIT RESUBMITTAL    |
|             | DATE: 8/9/2024<br>YHS<br>PERMIT RESUBMITTAL    |

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## Irrigation Storage Tank Water Balance

| Month     | Beginning Balance | Process Wastewater | Land Application Capacity | Ending Balance |
|-----------|-------------------|--------------------|---------------------------|----------------|
| January   | 29,700            | 13,500             | 0                         | 43,200         |
| February  | 43,200            | 13,500             | 0                         | 56,700         |
| March     | 56,700            | 13,500             | 0                         | 70,200         |
| April     | 70,200            | 10,800             | 0                         | 81,000         |
| May       | 81,000            | 10,800             | 65,216                    | 26,584         |
| June      | 26,584            | 13,500             | 163,039                   | 0              |
| July      | 0                 | 27,000             | 163,039                   | 0              |
| August    | 0                 | 48,600             | 97,823                    | 0              |
| September | 0                 | 48,600             | 97,823                    | 0              |
| October   | 0                 | 40,500             | 65,216                    | 0              |
| November  | 0                 | 16,200             | 0                         | 16,200         |
| December  | 16,200            | 13,500             | 0                         | 29,700         |
|           |                   | 270,000            | 652,155                   |                |

### Notes:

1. All values shown above for beginning balance, inflow, outflow and ending balance are in units of gallons.
2. See attached tables for detailed explanation of process wastewater and irrigation data presented in this table.
3. This water balance is based on the assumption that the tank is empty in August, just prior to crush.
4. This table is intended to illustrate waste disposal capability only. Where irrigation demand exceeds available treated wastewater availability additional irrigation water will be provided by another source.

## Winery Process Wastewater Generation Analysis

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|                                   |                                |
|-----------------------------------|--------------------------------|
| Annual Wine Production            | 45,000 gallons                 |
| Wastewater Generation Rate        | 6 gallons per gallon of wine   |
| Annual Wastewater Generation      | 270,000 gallons                |
| Crush Season Length               | 45 days                        |
| Wastewater Generated During Crush | 1.5 gallons per gallon of wine |
| Peak Wastewater Generation Rate   | 1,500 gallons per day          |

| Winery Process Wastewater Generation Table |                            |                        |                    |
|--|----------------------------|------------------------|--------------------|
| Month                                      | Percentage of Annual Total | Monthly Flow (gallons) | Average Flow (gpd) |
| January                                    | 5.0%                       | 13,500                 | 435                |
| February                                   | 5.0%                       | 13,500                 | 482                |
| March                                      | 5.0%                       | 13,500                 | 435                |
| April                                      | 4.0%                       | 10,800                 | 360                |
| May  | 4.0%                       | 10,800                 | 348                |
| June                                       | 5.0%                       | 13,500                 | 450                |
| July                                       | 10.0%                      | 27,000                 | 871                |
| August                                     | 18.0%                      | 48,600                 | 1,568              |
| September                                  | 18.0%                      | 48,600                 | 1,620              |
| October                                    | 15.0%                      | 40,500                 | 1,306              |
| November                                   | 6.0%                       | 16,200                 | 540                |
| December                                   | 5.0%                       | 13,500                 | 435                |
| Total                                      | 100.0%                     | 270,000                |                    |

### Notes:

- I. Wastewater generation rates and monthly proportioning are based on our past experience with similar projects.

## Irrigation Schedule Analysis

### Vineyard Information:

|                           |                                  |
|---------------------------|----------------------------------|
| Total acres of vines      | 4 acres                          |
| Vine Row Spacing (approx) | 7 feet                           |
| Vine Spacing (approx)     | 5 feet (estimated)               |
| Vine density              | 1,245 vines per acre (estimated) |
| Total Vine Count          | 4,978 vines                      |

### Irrigation Information:

Seasonal Irrigation<sup>1</sup> 131.0 gallons per vine (May through October)

Non-Irrigation Application 0 inches per month October through April

| Irrigation Schedule |                                 |                               |                               |   |                 |
|---------------------|---------------------------------|-------------------------------|-------------------------------|---|-----------------|
| Month               | Monthly Percentage <sup>2</sup> | Irrigation per Vine (gallons) | Seasonal Irrigation (gallons) | Non-Seasonal Irrigation Application (gallons) | Total (gallons) |
| January             |                                 | 0.0                           | 0                             | 0   | 0               |
| February            |                                 | 0.0                           | 0                             | 0   | 0               |
| March               |                                 | 0.0                           | 0                             | 0   | 0               |
| April               |                                 | 0.0                           | 0                             | 0   | 0               |
| May                 | 10%                             | 13.1                          | 65,216                        | 0   | 65,216          |
| June                | 25%                             | 32.8                          | 163,039                       | 0   | 163,039         |
| July                | 25%                             | 32.8                          | 163,039                       | 0   | 163,039         |
| August              | 15%                             | 19.7                          | 97,823                        | 0   | 97,823          |
| September           | 15%                             | 19.7                          | 97,823                        | 0   | 97,823          |
| October             | 10%                             | 13.1                          | 65,216                        | 0   | 65,216          |
| November            |                                 | 0.0                           | 0                             | 0   | 0               |
| December            |                                 | 0.0                           | 0                             | 0   | 0               |
| Total               | 100%                            | 131.0                         | 652,155                       | 0   | 652,155         |

### Notes:

1. Irrigation per vine is based on 0.5 acre-feet per acre of vines per WAA.
2. Monthly vineyard irrigation percentages are based on our past experience with projects of this type.
3. Non-Irrigation Application is for managing tank levels and assumes a maximum of 5 operational days per month based on historic weather data (Summit Engineering NBRID Capacity Study, 1996) and a saturated soil infiltration rate of 0.1 gallons per square foot per day uniformly over the entire area.

