

“G”

Water Availability Analysis

WATER AVAILABILITY ANALYSIS

80 CLEAR CREEK ROAD

80 Clear Creek Rd

Napa, CA 94558

APN 027-310-032, -043



TABLE OF CONTENTS

PROJECT OVERVIEW 1

EXISTING WATER DEMAND 2

 EXISTING DOMESTIC WATER DEMAND..... 2

 EXISTING LANDSCAPE IRRIGATION WATER DEMAND..... 2

PROPOSED WATER DEMAND 3

 PROPOSED DOMESTIC WATER DEMAND 3

 PROPOSED LANDSCAPE IRRIGATION WATER DEMAND..... 3

 TOTAL PROPOSED WATER DEMAND..... 4

TIER I ANALYSIS: ESTIMATED AVERAGE ANNUAL GROUNDWATER RECHARGE 5

 METHOD..... 5

 RESULTS..... 6

TIER II ANALYSIS: WELL INTERFERENCE 6

 METHOD..... 6

 RESULTS..... 7

TIER III ANALYSIS: GROUNDWATER/SURFACE WATER INTERACTION 8

CONCLUSION 8

LIST OF ENCLOSURES

- Enclosure A: Vicinity Map and Overall Site Plan
- Enclosure B: Well Completion Reports
- Enclosure C: Existing Landscape and Irrigation Plans for APN 027-310-032
- Enclosure D: Existing Landscape Water Demand Calculation for APN 027-310-043
- Enclosure E: Proposed Landscape Plans
- Enclosure F: Excerpt from Ludhorff & Scalmanini Report with Recharge Data
- Enclosure G: PRISM Rainfall Data from 2015–2024
- Enclosure H: Site Slope Analysis
- Enclosure I: Tier 2 Well Drawdown Calculations

PROJECT OVERVIEW

80 Clear Creek Road in Napa, CA, located on APNs 027-310-032 and -043, is applying for an administrative viewshed permit (P22-00182) to develop several proposed improvements to the residential site, including the construction of a tennis pavilion, a new three-bedroom ADU, and new underground spa and storage areas. The only proposed changes to the site that are anticipated to affect water usage are the addition of the ADU and modifications made to existing landscaping.

The residential site is located across two parcels, both of which are outside the Napa Valley Subbasin and the Milliken-Sarco-Tuluca (MST) area. APN 027-310-032 covers 123.07 acres and contains the existing residences as well as the proposed ADU. APN 027-310-043 covers 95.05 acres and contains the gate and entrance road to the property, along with minor landscaping. A vicinity map showing the location of the site within Napa County and an overall site plan showing the general layout of the project site are included in Enclosure A.

There are four existing wells on site, but only three are currently in use: Well A, Well B, and Well D. Wells A and B are located on APN 027-310-032 and supply all domestic water and landscape irrigation water for the parcel. Well D is located on APN 027-310-043 and provides landscape irrigation water for the minor landscaping on this parcel. Well C, located on APN 027-310-043, was drilled for a proposed residence which was never built and is currently non-operational. This inactive well is proposed to remain on site for potential future use, following the criteria listed in Napa County Code 13.12.460(B) and under the condition that no groundwater extracted from the well will be used to support the proposed developments of P22-00182. Available well completion reports for Wells A, B, and D are provided in Enclosure B. No well completion report is available for Well C. The existing wells and their capacities are summarized in Table 1.

Table 1: Existing well use, status, and capacities.

Source	Primary Use	Status	Pumping rate ¹ (gpm)
Well A	Domestic water and landscape irrigation water for APN 027-310-032	Active	25
Well B	Domestic water and landscape irrigation water for APN 027-310-032	Active	16
Well C	Non-operational; drilled for proposed residence which was never built. Proposed to remain inactive for potential future use.	Inactive	Unknown
Well D	Landscape irrigation water for APN 027-310-043	Active	16
Note: 1. Yield testing has not been performed for any of the existing wells. The listed pumping rates are the capacities of the installed well pumps.			

Summit Engineering has prepared the following Water Availability Analysis (WAA) in compliance with the Napa County 2015 WAA Guidance Document, the Application of Public Trust Doctrine to Projects Dependent on Groundwater dated January 2024, and the Interim Napa County Well Permit Standards and WAA Requirements dated January 2024. The purpose of this WAA is to demonstrate that the proposed project will

result in no net increase in groundwater usage, will have no adverse impact on existing or future neighboring wells, and will reduce harm relative to current conditions through modifications to the operation of existing project wells.

EXISTING WATER DEMAND

The existing water demands on the property include:

- Domestic water demand from:
 - The seven-bedroom main residence
 - A two-bedroom guest house
 - A covered pool
- Landscape irrigation

The estimated water demand for these uses is calculated in the following sections.

EXISTING DOMESTIC WATER DEMAND

Historical flow meter data is not available for the property, and therefore the existing water demand for the property is estimated in Table 2 following the Guidelines for Estimating Residential Water Use from the Napa County 2015 WAA Guidance Document. The resulting existing domestic water demand is 0.75 acre-feet per year (af/yr).

Table 2: Existing domestic water demand.

Use	Number	Units	Water Use Allocation ¹	Units	Total Water Use (af/yr)
Main residence	1	Primary residence	0.50	af/yr/residence	0.50
Guest house	1	Secondary residence	0.20	af/yr/residence	0.20
Pool with cover	1	Pool	0.05	af/yr/pool	0.05
Total					0.75
Note: 1. Water use allocations are taken from the Guidelines for Estimating Residential Water Use in Appendix B of the Napa County 2015 WAA Guidance Document.					

EXISTING LANDSCAPE IRRIGATION WATER DEMAND

The existing landscape on APN 027-310-032 consists primarily of drought-tolerant trees and shrubs surrounding the main residence and guest house, as well as two lawn areas to the east and northwest of the main residence. Existing landscape planting and irrigation plans for APN 027-310-032 are included in Enclosure C. The Water Efficient Landscape Worksheet included with the existing irrigation plans show an Estimated Total Water Use of 1,037,134 gallons per year, or approximately 3.18 af/yr.

Landscaping on APN 027-310-043 includes olive and ginkgo trees planted near the site's entrance gate. The irrigated area for these trees spans approximately 10,300 square feet (SF). The estimated irrigation demand for

these trees is calculated in Enclosure D, resulting in an estimated irrigation demand of 0.56 af/yr. The resulting total existing irrigation water demand for both parcels is 3.74 af/yr.

PROPOSED WATER DEMAND

The only proposed changes to the site that are anticipated to affect water usage are the addition of the ADU and modifications made to existing landscaping. In order to offset the increased groundwater demand resulting from the proposed ADU, it is proposed to modify the existing landscaping to reduce on-site landscape irrigation demand.

PROPOSED DOMESTIC WATER DEMAND

The proposed domestic water demand is estimated in Table 3 following the Guidelines for Estimating Residential Water Use from the Napa County 2015 WAA Guidance Document. The proposed ADU is anticipated to add a new 0.20 af/yr of water demand, increasing the total proposed domestic water demand to 0.95 af/yr. All domestic water demand will continue to be supplied by Wells A and B.

Table 3: Existing domestic water demand.

Use	Number	Units	Water Use Allocation ¹	Units	Total Water Use (af/yr)
Main residence	1	Primary residence	0.50	af/yr/residence	0.50
Guest house	1	Secondary residence	0.20	af/yr/residence	0.20
Proposed ADU	1	Secondary residence	0.20	af/yr/residence	0.20
Pool with cover	1	Pool	0.05	af/yr/pool	0.05
Total					0.95
Note: 1. Water use allocations are taken from the Guidelines for Estimating Residential Water Use in Appendix B of the Napa County 2015 WAA Guidance Document.					

PROPOSED LANDSCAPE IRRIGATION WATER DEMAND

The only new plantings in the proposed landscaping plans are the trees near the residences on APN 027-310-032 shown in the proposed landscaping plan included in Enclosure E. These drought-tolerant trees will require no irrigation once established. As such, no new irrigation lines will be installed in these areas, and the proposed trees are not anticipated to impact irrigation groundwater demand. In order to establish the trees, it is proposed to use spray irrigation using trucked water for the first year after planting, after which the trees will require no watering.

In order to offset the increased domestic water demand resulting from the proposed ADU, it is proposed to modify the existing lawn areas such that the landscape irrigation water demand on APN 027-310-032 is reduced by at least 0.20 af/yr. A water audit of the existing irrigation system performed by the site's landscape designer revealed that irrigation for the lawn northwest of the main residence is provided by four irrigation valves. Two of these valves were found to provide 2,160 gallons/week of irrigation water, with the other two

providing 1,080 gallons/week. Based on this data, if one of the higher-flow valves were removed along with the corresponding lawn area which it irrigates, annual irrigation demand would be reduced by 112,320 gallons per year, or approximately 0.34 af/yr. Thus, the required 0.20 af/yr reduction in irrigation demand to achieve no net increase in overall groundwater demand could be met by removing this portion of lawn.

Additional or alternative possibilities for reducing landscape irrigation water demand include replacing a larger portion of the existing lawn with less water-intensive landscaping or replacing existing MP rotator sprinkler heads with more efficient drip irrigation nozzles.

Although it has been demonstrated that there exists potential for greater water savings, the proposed water demand reduction is conservatively presented as being 0.20 af/yr, the minimum value required to offset the domestic water demand of the proposed ADU. No changes are proposed to the existing landscaping on APN 027-310-043, and therefore the resulting total proposed landscape irrigation demand on the project site is 3.54 af/yr. A breakdown of the existing and proposed landscape irrigation demand is shown in Table 4.

Table 4: Existing and proposed landscape irrigation demand.

Source of Demand	Existing (af/yr)	Proposed (af/yr)	Difference (af/yr)
Landscaping on APN 027-310-032	3.18	2.98	-0.20
Landscaping on APN 027-310-043	0.56	0.56	0.00
Total	3.74	3.54	-0.20

TOTAL PROPOSED WATER DEMAND

A comparison of existing and proposed water demand by use is shown in Table 5. The total existing and proposed water demand is 4.49 af/yr; the proposed project results in no net increase in water demand.

Table 5: Existing and proposed groundwater demand comparison.

Source of Demand	Existing (af/yr)	Proposed (af/yr)	Difference (af/yr)
Domestic water	0.75	0.95	+0.20
Landscape irrigation	3.74	3.54	-0.20
Total	4.49	4.49	0.00

A breakdown of the existing and proposed water demand on each groundwater well is shown in Table 6. Water from Wells A and B are combined into a single 20,000-gallon storage tank that feeds both landscape irrigation and the domestic water treatment system, and there is no historical meter data for the individual usage of each well. Therefore, the relative usage of each well is assumed to be proportional to the relative capacities of the installed well pumps for each well (25 gpm for Well A and 16 gpm for Well B). This results in Well A providing approximately 61% of the water for APN 027-310-032, with Well B providing the remaining 39%. There are no proposed changes to the water distribution system, so the relative usage of each well is not expected to change. The proposed project results in no net increase in water demand on any individual project well.

Table 6: Existing and proposed groundwater demand on each project well.

Source of Demand	Existing (af/yr)	Proposed (af/yr)	Difference (af/yr)
Well A¹			
Landscape irrigation	0.46	0.58	+0.12
Domestic water	1.94	1.82	-0.12
Total for Well A	2.40	2.40	0.00
Well B¹			
Landscape irrigation	0.29	0.37	+0.08
Domestic water	1.24	1.16	-0.08
Total for Well B	1.53	1.53	0.00
Well D			
Landscape irrigation	0.56	0.56	0.00
Total for Well D	0.56	0.56	0.00
Notes: 1. The relative usage of Wells A and B is assumed to be proportional to the relative capacities of the installed well pumps for these wells (25 gpm and 16 gpm, respectively).			

TIER I ANALYSIS: ESTIMATED AVERAGE ANNUAL GROUNDWATER RECHARGE

Per the January 2024 Interim Napa County Well Permit Standards and WAA Requirements, projects outside of the Napa Valley Subbasin and the Milliken-Sarco-Tuluca area are required to estimate the average annual recharge occurring on the project parcel and consider this recharge relative to the estimated project water use.

METHOD

Luhdorff & Scalmanini Consulting Engineers (LSCE) previously prepared a Hydrogeologic Conceptualization and Characterization of Conditions for Napa County regarding the groundwater and hydrogeology of the Napa Valley, including a detailed study of the anticipated rainfall recharge in several watersheds. An excerpt of this report is provided in Enclosure F. LSCE used a mass balance approach to provide groundwater recharge estimates for several watersheds within Napa County. The project APNs 027-310-032 and 027-310-043 are located within the Dry Creek watershed, which LSCE estimates to have an average annual recharge rate equal to 6% of annual precipitation.

The average annual precipitation for the site was taken to be 37.21 inches/year based on the last ten years (2015–2024) of precipitation data from PRISM. The PRISM precipitation data set is provided in Enclosure G.

Per the LSCE report, recharge potential is significantly reduced for areas of land with a slope greater than 30 degrees (approximately 57% slope). As such, areas of the site with slopes greater than 57% are excluded from the parcel acreage when calculating recharge. A site slope analysis (see Enclosure H) was prepared for each parcel based on digital elevation models downloaded from the USGS National Map. The slope analysis revealed a total of approximately 52.08 acres of land sloped less than 57% on APN 027-310-032 and approximately

37.12 acres sloped less than 57% on APN 027-310-043. Therefore, the total land area available for recharge over the two parcels is approximately 89.20 acres.

Utilizing the watershed's rainfall recharge fraction of 6% (according to the LSCE groundwater characterization), the anticipated annual recharge for a typical year is calculated to be 16.60 af/yr:

Typical Annual Recharge

$$\begin{aligned} &= \left(\text{Available recharge area (acres)} * \text{Precipitation} \left(\frac{\text{ft}}{\text{year}} \right) * \text{Recharge Fraction (\%)} \right) \\ &= \left(89.20 \text{ acres} * \frac{37.12 \frac{\text{in}}{\text{year}}}{12 \frac{\text{in}}{\text{ft}}} * 6\% \right) \\ &= 16.60 \frac{\text{acre-ft}}{\text{year}} \end{aligned}$$

RESULTS

The estimated proposed project water use is 4.49 af/yr during both normal and dry water years. Using the estimated annual recharge of 16.60 af/yr, the proposed project water use is only 27.9% of the estimated average annual recharge. As the proposed project water use is less than the estimated average annual recharge and the project demonstrates no net increase in groundwater demand, the project complies with the 2024 Interim Napa County Well Permit Standards and WAA Requirements.

TIER II ANALYSIS: WELL INTERFERENCE

Per the 2024 Interim Napa County Well Permit Standards and WAA Requirements, a Tier 2 analysis of an existing well is required only for projects that demonstrate a net increase in groundwater use, and therefore no Tier 2 analysis is required for the proposed project. Nevertheless, a Tier 2 analysis was performed to demonstrate that there is no potential for any of the project wells to have an adverse effect on any existing or future neighboring wells.

Based on available data, there is only one neighboring well located within 500 feet of the existing project wells: the well located on APN 027-310-026 is located approximately 157 feet from Well D. Regardless, the analysis was performed for all three active wells, to cover any possibility of an existing or future neighboring well falling within a 500 ft range of the project wells.

METHOD

Using the Theis equation as indicated in the WAA Napa County guidelines, the groundwater drawdown from all property wells to the edge of the parcel was determined. The assumed closest distance that any neighboring well could be located is the edge of the parcel. Due to the limited data on the aquifer, assumed values that would yield a conservative drawdown estimate were selected from Napa County Water Availability Analysis guidelines:

- Hydraulic Conductivity very low range of 10 to 30 ft/day for project site (Water Availability Analysis Guidance Document Figure F-3)

- Specific Storage range of 1.5×10^{-5} to 3.1×10^{-4} (1/ft) (Water Availability Analysis Guidance Document Table F3)
- Continuous pumping (i.e., > 1 day) at stated flow rate.

The Theis equation can be seen below along with an example calculation.

$$\text{Theis Equation: Drawdown} = \frac{\text{Flow}}{(4\pi \times \text{Transmissivity})} \times W(u)$$

$$W(u) = \int_u^{\infty} \frac{1}{\omega} e^{-\omega} d\omega$$

$$u = \frac{(\text{Distance}^2 \times \text{Specific Storage})}{(4 \times \text{Transmissivity} \times \text{Time})}$$

$$\text{Transmissivity} = \text{Hydraulic Conductivity} \times \text{Aquifer Thickness}$$

Example for Well A drawdown effect on possible wells on adjacent properties:

$$u = \frac{(200 \text{ ft})^2 \times (1.50 \times 10^{-5})}{4 \times 10 \frac{\text{ft}}{\text{day}} \times 140 \text{ ft} \times 1 \text{ day}} = 1.07 \times 10^{-4}$$

With this value of u , $W(u) = 8.58$

$$\text{Drawdown} = \frac{25 \frac{\text{gal}}{\text{min}} \times 0.1337 \frac{\text{cuft}}{\text{gal}} \times 1,440 \frac{\text{min}}{\text{day}}}{4\pi \times 10 \frac{\text{ft}}{\text{day}} \times 140 \text{ ft}} \times 8.58 = 2.35 \text{ ft}$$

Table 7 shows a summary of the worst-case scenario drawdown results for the active parcel wells. More detailed tables can be found in Enclosure I.

Table 7: Well drawdown calculation results.

Well	Well Flow Rate (gpm)	Distance to Property Line (ft)	Aquifer Thickness (ft)	Estimated Drawdown (ft)
Well A	25	200	140	2.35
Well B	16	470	120	1.37
Well D	16	35	100	2.87

RESULTS

Using very conservative estimates for specific storage and hydraulic conductivity, no project well produces a drawdown greater than 10 feet on any existing or future wells that could be adjacent to the property. The Water Availability Analysis guidelines establish a 10-foot drawdown as the default criteria to determine significant adverse effects. Since the estimated drawdown of the well is less than 10 feet, no significant impact

is expected to existing or future wells on adjacent parcels. Furthermore, the proposed project results in no net increase in groundwater use, meaning the project complies with the 2024 Interim Napa County Well Permit Standards and WAA Requirements irrespective of the project wells' impact on neighboring wells.

TIER III ANALYSIS: GROUNDWATER/SURFACE WATER INERACTION

Per the Interim Napa County Well Permit Standards and WAA Requirements dated January 2024, projects modifying the use of an existing well within 1,500 feet of a significant stream inside the Napa River watershed are not required to conduct a Tier 3 analysis if modifications to the location, construction, or operations of the project wells are made to reduce harm relative to current conditions.

While Well B is located outside of the 1,500 significant stream buffer, Wells A and D are located approximately 668 feet and 157 feet from significant streams, respectively. As part of the proposed project, it is proposed to install flow restrictors on Wells A and D to reduce the pumping rates of these wells, thereby reducing any potential impact these wells may currently have on the nearby significant streams. As this proposed change in operation will reduce harm relative to current conditions, a Tier 3 analysis is not required.

CONCLUSION

The only proposed changes in use at 80 Clear Creek Road that will affect groundwater demand is the construction of a new ADU and proposed modifications to existing landscaping. The increased water demand resulting from the proposed ADU is expected to be offset by these landscape modifications, resulting in no net increase in groundwater demand: Both the existing and proposed estimated water demand are 4.49 af/yr. This no net increase in groundwater demand demonstrates that the project complies with the Tier 1 and Tier 2 requirements published in the 2024 Interim Napa County Well Permit Standards and WAA Requirements. Furthermore, the proposed estimated water demand of 4.49 af/yr is significantly less than the site's estimated annual groundwater recharge of 16.60 af/yr, and none of the project wells produce a drawdown greater than 10 feet on any existing or future wells that could be adjacent to the property. Well B is located greater than 1,500 feet from any significant streams and therefore is not subject to a Tier 3 analysis. It is proposed to install flow restrictors on Wells A and D in order to reduce the pumping rates of these wells, thereby reducing the potential harm of each well relative to current conditions.

80 Clear Creek Road
Water Availability Analysis
July 11, 2025

SUMMIT ENGINEERING, INC.
Project No.: 2021275

ENCLOSURE A

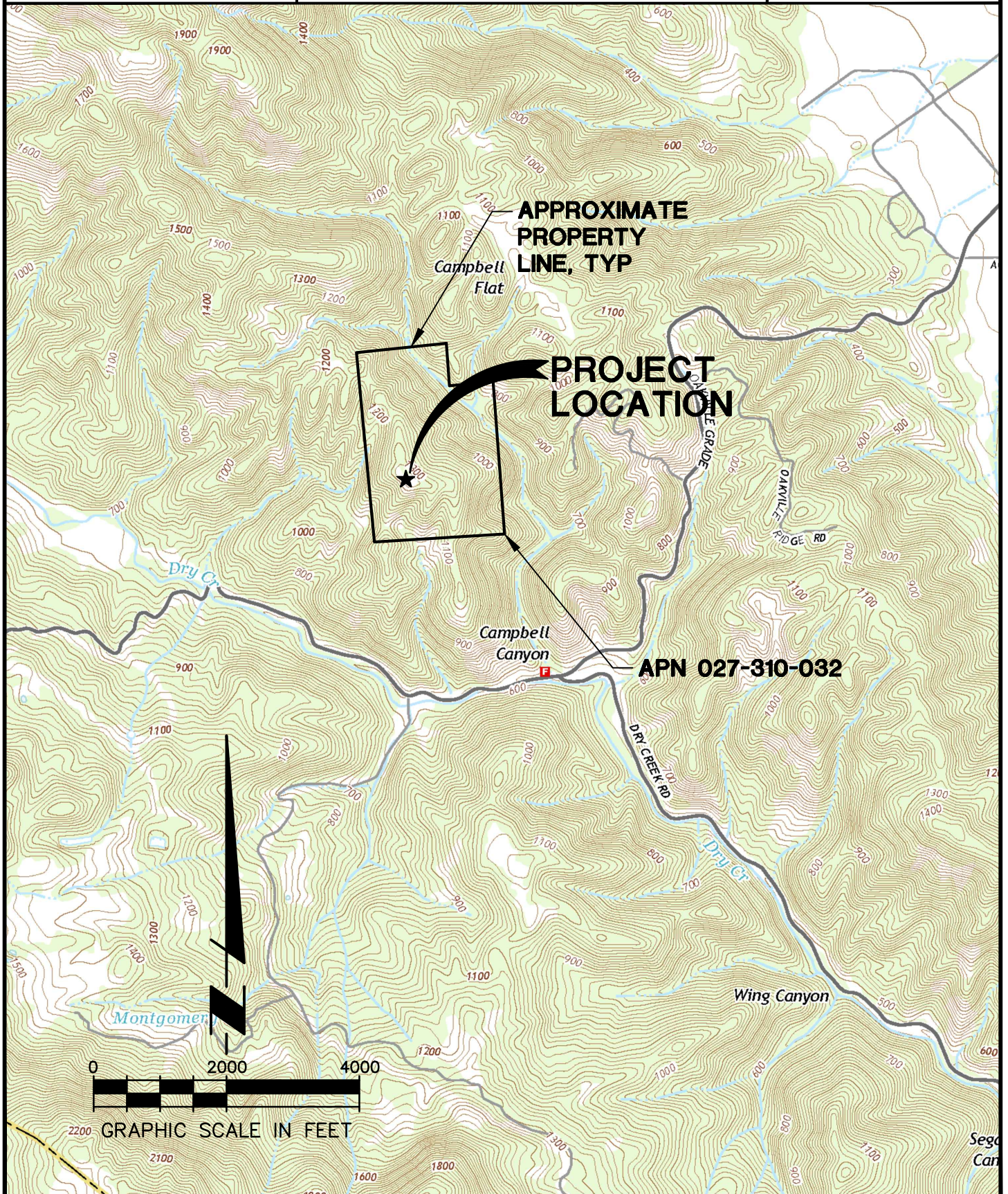
VICINITY MAP AND OVERALL SITE PLAN

S U M M I T



PONDEROSA ONE LLC
80 CLEAR CREEK ROAD
NAPA, CA
APN 027-310-032
VICINITY MAP

PROJECT NO. 2021275
DATE 2021-06-29
SHT NO 1 OF 1
BY MB CHK MS



0 200 400

GRAPHIC SCALE IN FEET

APN 027-310-032

**APPROXIMATE LOCATION
OF PROPOSED PRIMARY
AND RESERVE DRIPFIELD
DISPOSAL AREAS**

**FILL PLACEMENT AREA,
SEE SHEET C4.4**

APN 027-310-043

AC	ASPHALT CONCRETE
AD	AREA DRAIN
ADU	ACCESSORY DWELLING UNIT
APN	ASSESSOR'S PARCEL NUMBER
BLDG	BLDG
CL	CENTERLINE
CONF	CONFORM
CONC	CONCRETE
DI	DRAIN INLET
(E)	EXISTING
EG	EXISTING GROUND
FG	FINISH GRADE
FH	FIRE HYDRANT
FS	FINISH SURFACE
FL	FLOWLINE
GAL	GALLON
GB	GRADE BREAK
*HP	HIGH POINT
NCRSS	NAPA COUNTY ROAD & STREET STANDARDS
PD	PLANTER DRAIN
PIP	PROTECT IN PLACE
SB	SETBACK
SF	SQUARE FEET
STA	STATION
SD	STORM DRAIN
SS	SANITARY SEWER
STD	STANDARD
TC	TOP OF CONCRETE
TCC	TOP OF CONCRETE CURB
TD	TRENCH DRAIN
TI	TRAFFIC INDEX
TP	TOP OF PAVEMENT
TOW	TOP OF WALL
TYP	TYPICAL
UNO	UNLESS OTHERWISE NOTED
USP	UNDER SEPARATE PERMIT

VALUES ARE FOR PERMITTING ONLY AND ARE NOT TO BE USED FOR BIDDING.
CONTRACTOR SHALL PERFORM THEIR OWN EARTHWORK CALCULATIONS.

LIMITS OF GRADING: 1.6 ACRES

CUT VOLUME:	8,241 CU. YDS.
FILL VOLUME:	2,327 CU. YDS.
NET VOLUME:	5,914 CU. YDS. (CUT)

NOTES:

REFER TO CONCEPTUAL GRADING PLANS FOR SITE DEVELOPMENT.



SUMMIT
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707-527-0775 • www.summit-sr.com

PONDEROSA ONE LLC
80 CLEAR CREEK ROAD
NAPA, CA 94558
APN 027-310-032 & 043

VIEWSHED PERMIT

OVERALL SITE PLAN

#	DATE	DESCRIPTION
	9/07/2022	PERMIT SUBMITT
1	10/3/2023	PLAN CHECK RESPONSE
2	3/29/2024	PLAN CHECK RESPONSE
3	2/28/2025	PLAN CHECK RESPONSE
4	5/07/2025	PLAN CHECK RESPONSE
	12/04/2025	PLAN CHECK RESPONSE

JOB NO:	20212
SCALE:	AS SHOWN
DRAWN:	
CHECKED:	
SHEET	

C2.0
SHEET 1 OF 1

80 Clear Creek Road
Water Availability Analysis
July 11, 2025

SUMMIT ENGINEERING, INC.
Project No.: 2021275

ENCLOSURE B

WELL COMPLETION REPORTS

ORIGINAL
File with DWR

Well A

STATE OF CALIFORNIA
WELL COMPLETION REPORT

Refer to Instruction Pamphlet

No. **e0133552**

Page 1 of 1

Owner's Well No. **2-2011**

Date Work Began **8/3/2011**, Ended **8/8/2011**

Local Permit Agency **Napa County Environmental Mgmt**

Permit No. **E11-00267**

Permit Date **7/1/2011**

DWR USE ONLY -- DO NOT FILL IN

STATE WELL NO./STATION NO.	
LATITUDE	LONGITUDE
APN/TRS/OTHER	

GEOLOGIC LOG

ORIENTATION (✓) ☒ VERTICAL ☐ HORIZONTAL ☐ ANGLE (SPECIFY)

DEPTH FROM SURFACE
FL. to FL.

DRILLING METHOD **ROTARY**

FLUID **AIR**

DESCRIPTION

Describe material, grain, size, color, etc.

WELL OWNER

Name **[REDACTED]**

Mailing Address **[REDACTED]**

City **[REDACTED]** State **CA** ZIP **[REDACTED]**

WELL LOCATION

Address **80 Clear Creek Road**

City **Napa** CA

County **Napa**

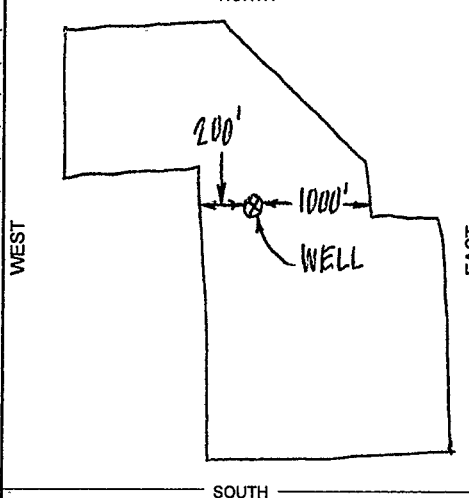
APN Book **027** Page **310** Parcel **032**

Township **[REDACTED]** Range **[REDACTED]** Section **[REDACTED]**

Latitude **[REDACTED]** DEG. MIN. SEC.

LOCATION SKETCH

NORTH



Illustrate or Describe Distance of Well from Roads, Buildings, Fences, Rivers, etc. and attach a map. Use additional paper if necessary. PLEASE BE ACCURATE & COMPLETE.

ACTIVITY (✓)

☒ NEW WELL

MODIFICATION/REPAIR

☐ Deepen

☐ Other (Specify)

☐ DESTROY (Describe Procedures and Materials Under "GEOLOGIC LOG")

PLANNED USES (✓)

WATER SUPPLY

☒ Domestic ☐ Public
☒ Irrigation ☐ Industrial

MONITORING ☐

TEST WELL ☐

CATHODIC PROTECTION ☐

HEAT EXCHANGE ☐

DIRECT PUSH ☐

INJECTION ☐

VAPOR EXTRACTION ☐

SPARGING ☐

REMEDIATION ☐

OTHER (SPECIFY) ☐

WATER LEVEL & YIELD OF COMPLETED WELL

DEPTH TO FIRST WATER **230** (Ft.) BELOW SURFACE

1

DEPTH OF STATIC WATER LEVEL **187** (Ft.) & DATE MEASURED **8/8/2011**

ESTIMATED YIELD **60** (GPM) & TEST TYPE **AIR LIFT**

TEST LENGTH **2** (Hrs.) TOTAL DRAWDOWN **N/A** (Ft.)

May not be representative of a well's long-term yield.

TOTAL DEPTH OF BORING **460** (Feet)

TOTAL DEPTH OF COMPLETED WELL **378** (Feet)

DEPTH FROM SURFACE			BORE - HOLE DIA. (Inches)	CASING (S)					DEPTH FROM SURFACE			ANNULAR MATERIAL				
				TYPE (✓)				MATERIAL / GRADE				INTERNAL DIAMETER (Inches)	GAUGE OR WALL THICKNESS	SLOT SIZE IF ANY (Inches)	TYPE	
FL.	to	FL.		BLANK	SCREEN	CON-DUCTOR	FILL PIPE		CE-MENT (✓)	BEN-TONITE (✓)	FILL (✓)				FILTER PACK (TYPE/SIZE)	
0		25	12													
25		460	9													
0		198		✓			PVC F480	6	SDR-21							
198		338			✓		PVC F480	6	SDR-21						CONCRETE CHIPS	
338		378		✓			PVC F480	6	SDR-21				✓		PEA GRAVEL	

ATTACHMENTS (✓)

- ☐ Geologic Log
- ☐ Well Construction Diagram
- ☐ Geophysical Log(s)
- ☐ Soil/Water Chemical Analysis
- ☐ Other

ATTACH ADDITIONAL INFORMATION, IF IT EXISTS.

CERTIFICATION STATEMENT

I, the undersigned, certify that this report is complete and accurate to the best of my knowledge and belief.

NAME **HUCKFELDT WELL DRILLING, INC.**

(PERSON, FIRM, OR CORPORATION) (TYPED OR PRINTED)

2110 Penny Lane

ADDRESS

Napa

CITY

CA

STATE

94559

ZIP

Signed

WELL DRILLER/AUTHORIZED REPRESENTATIVE

08/12/11

DATE SIGNED

439-746

C-57 LICENSE NUMBER

ORIGINAL
File with DWR

Well B

STATE OF CALIFORNIA
WELL COMPLETION REPORT
Refer to Instruction Pamphlet

DWR USE ONLY -- DO NOT FILL IN

STATE / WELL NO. / STATION NO. 10

LATITUDE 38 LONGITUDE 121

APN/TRS/OTHER

Page 1 of 1

Owner's Well No. 2-09

No. **e0102663**

Date Work Began 11/24/2009, Ended 12/2/2009

Local Permit Agency Napa County Environmental Mgmt

Permit No. E09-00461

Permit Date 10/15/2009

GEOLOGIC LOG

WELL OWNER

ORIENTATION (✓) ☒ VERTICAL ☐ HORIZONTAL ☐ ANGLE (SPECIFY)

DRILLING METHOD ROTARY FLUID AIR

DEPTH FROM SURFACE
Ft. to Ft.

DESCRIPTION

Describe material, grain, size, color, etc.

DEPTH FROM SURFACE Ft. to Ft.	DESCRIPTION
0 to 65	BROWN SHALE & CLAY
65 to 215	50% SHALE / 50% SILTSTONE
215 to 310	30% SHALE / 70% SILTSTONE
310 to 370	70% SHALE / 30% SILTSTONE
370 to 500	50% SHALE / 50% SILTSTONE

Name [REDACTED]

Mailing Address [REDACTED]

CA

CITY

STATE

ZIP

WELL LOCATION

Address Dry Creek Road

City Napa CA

County Napa

APN Book Q27 Page 310 Parcel 032

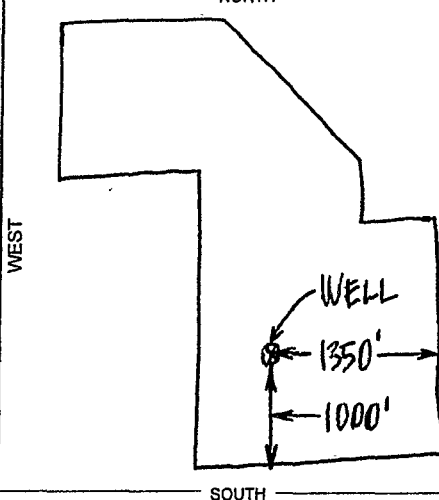
Township _____ Range _____ Section _____

Latitude

DEG. MIN. SEC.

LOCATION SKETCH

NORTH



Illustrate or Describe Distance of Well from Roads, Buildings, Fences, Rivers, etc. and attach a map. Use additional paper if necessary. PLEASE BE ACCURATE & COMPLETE.

DEG. MIN. SEC.

ACTIVITY (✓)

☒ NEW WELL

MODIFICATION/REPAIR

Deepen

Other (Specify)

DESTROY (Describe Procedures and Materials Under "GEOLOGIC LOG")

PLANNED USES (✓)

WATER SUPPLY

☒ Domestic ☐ Public

Imigation ☐ Industrial

MONITORING ☐

TEST WELL ☐

CATHODIC PROTECTION ☐

HEAT EXCHANGE ☐

DIRECT PUSH ☐

INJECTION ☐

VAPOR EXTRACTION ☐

SPARGING ☐

REMEDIATION ☐

OTHER (SPECIFY) _____

WATER LEVEL & YIELD OF COMPLETED WELL

DEPTH TO FIRST WATER 215 (Ft.) BELOW SURFACE

1

DEPTH OF STATIC WATER LEVEL 150 (Ft.) & DATE MEASURED 12/2/2009

ESTIMATED YIELD 23 (GPM) & TEST TYPE AIR LIFT

TEST LENGTH 3 (Hrs.) TOTAL DRAWDOWN N/A (Ft.)

May not be representative of a well's long-term yield.

TOTAL DEPTH OF BORING 500 (Feet)

TOTAL DEPTH OF COMPLETED WELL 338 (Feet)

DEPTH FROM SURFACE			BORE-HOLE DIA. (Inches)	CASING (S)							DEPTH FROM SURFACE			ANNULAR MATERIAL				
				TYPE (✓)				MATERIAL / GRADE	INTERNAL DIAMETER (Inches)	GAUGE OR WALL THICKNESS				SLOT SIZE IF ANY (Inches)	TYPE			
Fl.	to	Fl.	BLANK	SCREEN	CON- DUCTOR	FILL PIPE									Fl.	to	Fl.	CE- MENT (✓)
0	25		12								0	3		✓			CONCRETE CHIPS	
25	500		9								3	22			✓			
											22	500				✓	PEA GRAVEL	
0	198			✓				PVC F480	6	SDR-21								
198	318				✓			PVC F480	6	SDR-21							.032	
318	338			✓				PVC F480	6	SDR-21								

ATTACHMENTS (✓)

Geologic Log
Well Construction Diagram
Geophysical Log(s)
Soil/Water Chemical Analysis
Other

ATTACH ADDITIONAL INFORMATION, IF IT EXISTS.

CERTIFICATION STATEMENT

I, the undersigned, certify that this report is complete and accurate to the best of my knowledge and belief.

NAME HUCKFELDT WELL DRILLING, INC.

(PERSON, FIRM, OR CORPORATION) (TYPED OR PRINTED)

2110 Penny Lane

ADDRESS

Napa

CITY

CA

STATE

94559

ZIP

Signed [Signature]

WELL DRILLER/AUTHORIZED REPRESENTATIVE

12/14/09

DATE SIGNED

439-746

C-57 LICENSE NUMBER

ORIGINAL
File with DWR

Well D

STATE OF CALIFORNIA
WELL COMPLETION REPORT
Refer to Instruction Pamphlet

DWR USE ONLY — DO NOT FILL IN

STATE WELL NO./STATION NO.

LATITUDE LONGITUDE

APN/TRS/OTHER

Page 1 of 1

Owner's Well No. 1

No. **e039617**

Date Work Began 5/15/2006, Ended 5/18/2006

Local Permit Agency Napa County Environmental Mgmt

Permit No. 96-10390 Permit Date 9/30/1998

GEOLOGIC LOG

WELL OWNER

ORIENTATION (✓) ☒ VERTICAL ☐ HORIZONTAL ☐ ANGLE (SPECIFY)

DEPTH FROM SURFACE
Ft. to Ft.

DRILLING METHOD **ROTARY** FLUID **AIR**

DESCRIPTION
Describe material, grain, size, color, etc.

0	3	BROWN CLAY
3	135	HARD SHALE
135	137	SANDSTONE
137	150	HARD SHALE
150	152	SANDSTONE
152	160	HARD SHALE
160	162	SANDSTONE
162	180	90% HARD SHALE/ 10% SANDSTONE
180	200	50% HARD SHALE/ 50% SANDSTONE
200	300	HARD SHALE
CONTINUED CASING LAYOUT		
170	250	SCREEN PVC 6" .032 SLOT
250	270	BLANK PVC 6"
270	290	SCREEN PVC 6" .032 SLOT

Name **C**

Mailing Address **60 Campbell Creek Road**

CITY **Napa** STATE **CA** ZIP **94559**

WELL LOCATION

Address **60 Campbell Creek Road**

City **Napa** County **Napa**

APN Book **027** Page **310** Parcel **031**

Township Range Section

Latitude DEG. MIN. SEC.

LOCATION SKETCH

NORTH

WEST EAST

WELL 35' 140'

DAKVILLE GRADE RD.

SOUTH

Illustrate or Describe Distance of Well from Roads, Buildings, Fences, Rivers, etc. and attach a map. Use additional paper if necessary. PLEASE BE ACCURATE & COMPLETE.

ACTIVITY (✓)

☒ NEW WELL

MODIFICATION/REPAIR

Deepen

Other (Specify)

DESTROY (Describe Procedures and Materials Under "GEOLOGIC LOG")

PLANNED USES (✓)

WATER SUPPLY

☒ Domestic ☐ Public

☐ Irrigation ☐ Industrial

MONITORING

TEST WELL

CATHODIC PROTECTION

HEAT EXCHANGE

DIRECT PUSH

INJECTION

VAPOR EXTRACTION

SPARGING

REMEDIATION

OTHER (SPECIFY)

RECEIVED

JUN 7 2006

DEPT. OF
ENVIRONMENTAL MANAGEMENT

WATER LEVEL & YIELD OF COMPLETED WELL

DEPTH TO FIRST WATER **60** (Ft.) BELOW SURFACE

1

DEPTH OF STATIC WATER LEVEL **15** (Ft.) & DATE MEASURED **5/18/2006**

ESTIMATED YIELD **25** (GPM) & TEST TYPE **AIR LIFT**

TEST LENGTH **2** (Hrs.) TOTAL DRAWDOWN **N/A** (Ft.)

May not be representative of a well's long-term yield.

DEPTH FROM SURFACE			BORE - HOLE DIA. (Inches)	CASING (S)					DEPTH FROM SURFACE		ANNULAR MATERIAL						
				TYPE (✓)				MATERIAL / GRADE			INTERNAL DIAMETER (Inches)	GAUGE OR WALL THICKNESS	SLOT SIZE IF ANY (Inches)	TYPE			
Ft.	to	Ft.	BLANK	SCREEN	CON- DUCTOR	FILL PIPE									Ft.	to	Ft.
0		25	12								0		5	✓			CONCRETE
25		300	9								5		25		✓		CHIPS
											25		290			✓	PEA GRAVEL
0		50		✓				PVC F480	6	SDR-21							
50		150			✓			PVC F480	6	SDR-21							.032
150		170		✓				PVC F480	6	SDR-21							

ATTACHMENTS (✓)

- Geologic Log
- Well Construction Diagram
- Geophysical Log(s)
- Soil/Water Chemical Analysis
- Other

ATTACH ADDITIONAL INFORMATION, IF IT EXISTS.

CERTIFICATION STATEMENT

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05/26/06

DATE SIGNED

439-746

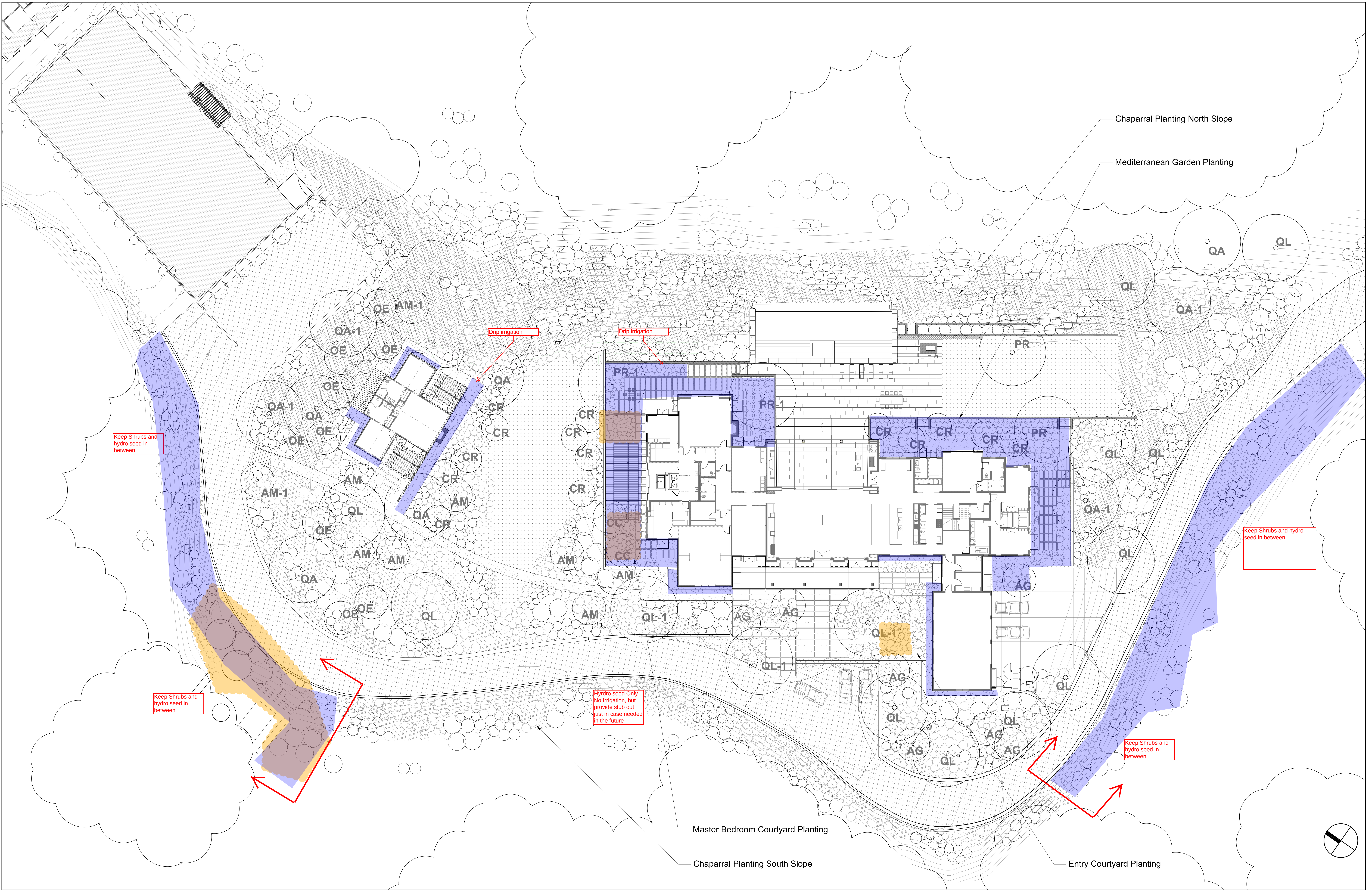
C-57 LICENSE NUMBER

80 Clear Creek Road
Water Availability Analysis
July 11, 2025

SUMMIT ENGINEERING, INC.
Project No.: 2021275

ENCLOSURE C

EXISTING LANDSCAPE AND IRRIGATION PLANS FOR APN 027-310-032



Consultant:

Stamp:

Project Name:

**Rancho
Arroyo
Claro**

**80 Clear Creek Rd.
Napa, CA**

Sheet Title:

**Detailed Irrigation
Plan**

Submittal:	Date:
Conformed Pricing Set	11/11/13
Permit Set	12/09/13
100% CD Set	01/31/14
100% CD Planting Set	02/07/14
100% CD Set - REV3	02/20/14

Revision: Date:

Scale: 1/16" = 1' - 0"

Sheet: **L7.1**

NOTES:
1. UNLABELED LATERAL LINE PIPE DOWN STREAM FROM SIZED PIPE IS TO BE 3/4" IN SIZE.

2. SPRINKLER HEADS ADJACENT TO BUILDINGS ARE TO BE PLACED A MINIMUM OF 12" FROM BUILDING WALLS.

3. REFER TO HUNTER INDUSTRIES SPECIFICATIONS FOR GROUNDING EQUIPMENT FOR TWO-WIRE DECODER CONTROL SYSTEM. PROVIDE GROUNDING FOR EVERY EIGHT (8) REMOTE CONTROL VALVES. GROUNDING SHALL BE INSTALLED EVERY 500 FEET OR CLOSER AS VALVE COUNT REQUIRES. GROUNDING SHALL BE TESTED AND MUST ACHIEVE HUNTER INDUSTRIES MINIMUM STANDARDS FOR EFFECTIVE GROUNDING.

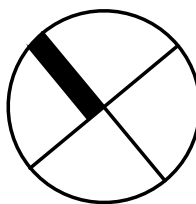
GROUNDING PLATE LOCATION FOR GROUNDING IRRIGATION TWO-WIRE PATH. SEE DETAILS AND MANUFACTURER'S INSTRUCTIONS. (TYPICAL)

BUBBLER LATERAL PIPE SIZING

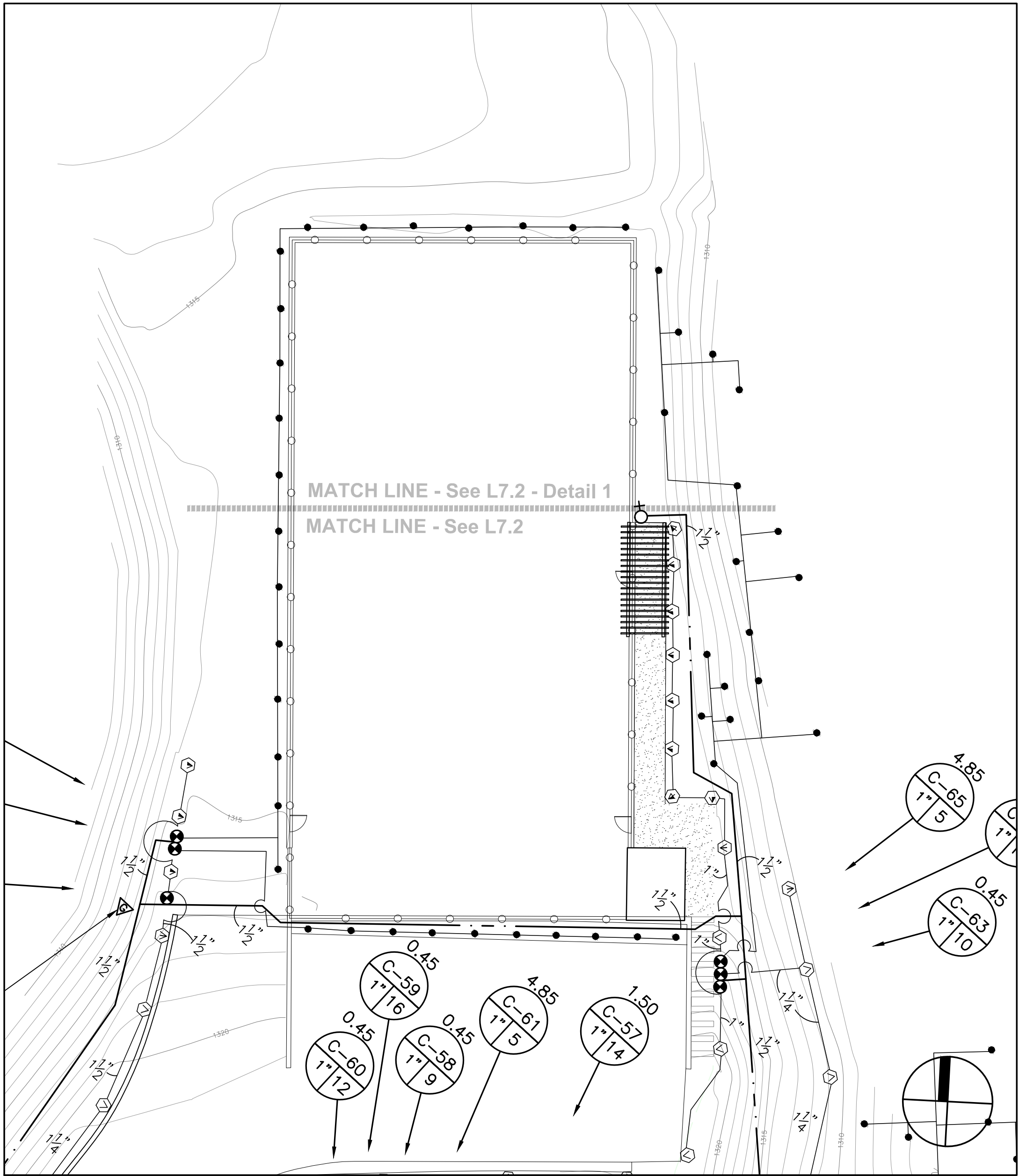
QTY OF BUBBLERS	LINE SIZE
1 - 10	3/4"
11 - 20	1"
21 - 35	1 1/4"
36 - 60	1 1/2"
61 +	2"

DICKSON & ASSOCIATES, INC.
LANDSCAPE IRRIGATION

MARTY DICKSON, ASIC-PIG
TEL(530) 547-5515 FAX(530) 547-5513
P.O. BOX 415
PALO CEDRO, CALIFORNIA 96073
© Dickson & Associates, Inc.



- NOTES:
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1 Irrigation Layout Detail @ Tennis Court

SCALE: 1/16" = 1'-0"

BUBBLER LATERAL PIPE SIZING

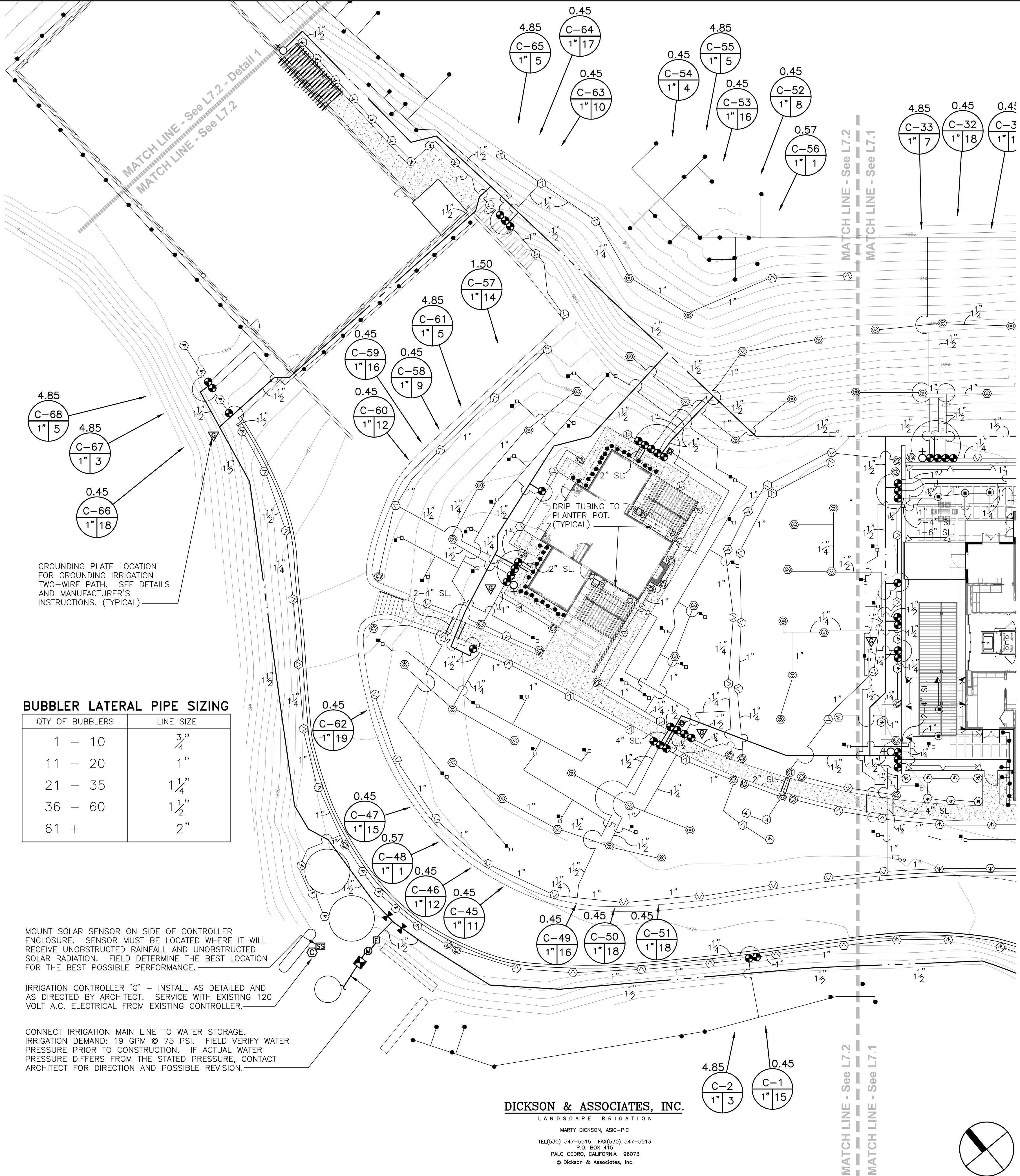
QTY OF BUBBLERS	LINE SIZE
1 - 10	3/4"
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36 - 60	1 1/2"
61 +	2"

MOUNT SOLAR SENSOR ON SIDE OF CONTROLLER ENCLOSURE. SENSOR MUST BE LOCATED WHERE IT WILL RECEIVE UNOBSTRUCTED RAINFALL AND UNOBSTRUCTED SOLAR RADIATION. FIELD DETERMINE THE BEST LOCATION FOR THE BEST POSSIBLE PERFORMANCE.

IRRIGATION CONTROLLER 'C' - INSTALL AS DETAILED AND AS DIRECTED BY ARCHITECT. SERVICE WITH EXISTING 120 VOLT A.C. ELECTRICAL FROM EXISTING CONTROLLER.

CONNECT IRRIGATION MAIN LINE TO WATER STORAGE. IRRIGATION DEMAND: 19 GPM @ 75 PSI. FIELD VERIFY WATER PRESSURE PRIOR TO CONSTRUCTION. IF ACTUAL WATER PRESSURE DIFFERS FROM THE STATED PRESSURE, CONTACT ARCHITECT FOR DIRECTION AND POSSIBLE REVISION.

GROUNDING PLATE LOCATION FOR GROUNDING IRRIGATION TWO-WIRE PATH. SEE DETAILS AND MANUFACTURER'S INSTRUCTIONS. (TYPICAL)



DICKSON & ASSOCIATES, INC.

LANDSCAPE IRRIGATION

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PALO CEDRO, CALIFORNIA 96073
© Dickson & Associates, Inc.

LUTSKO ASSOCIATES
landscape

2815 18th Street
San Francisco, CA 94110-2002
t: 415.920.2800 f: 415.920.2809

Consultant:

Stamp:

Project Name:

**Rancho
Arroyo
Claro**

**80 Clear Creek Rd.
Napa, CA**

Sheet Title:

**Detailed Irrigation
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100% CD Planting Set	02/07/14
100% CD Set - REV3	02/20/14

Revision:	Date:
-----------	-------

Scale: 1/16" = 1' - 0"

Sheet:

L7.2

IRRIGATION NOTES

1. THESE IRRIGATION DRAWINGS ARE DIAGRAMMATIC AND INDICATIVE OF THE WORK TO BE INSTALLED. ALL PIPING, VALVES, ETC. SHOWN WITHIN PAVED AREAS IS FOR CLARITY ONLY AND ARE TO BE INSTALLED WITHIN PLANTING AREAS WHERE POSSIBLE. DUE TO THE SCALE OF THE DRAWINGS, IT IS NOT POSSIBLE TO INDICATE ALL OFFSETS, FITTINGS, SLEEVES, ETC., WHICH MAY BE REQUIRED. THE CONTRACTOR IS REQUIRED TO INVESTIGATE THE STRUCTURAL AND FINISHED CONDITIONS AFFECTING ALL OF THE CONTRACT WORK INCLUDING OBSTRUCTIONS, GRADE DIFFERENCES OR AREA DIMENSIONAL DIFFERENCES WHICH MAY NOT HAVE BEEN CONSIDERED IN THE ENGINEERING. IN THE EVENT OF FIELD DIFFERENCES, THE CONTRACTOR IS REQUIRED TO PLAN THE INSTALLATION WORK ACCORDINGLY BY NOTIFICATION AND APPROVAL OF THE OWNER'S AUTHORIZED REPRESENTATIVE AND ACCORDING TO THE CONTRACT SPECIFICATION. THE CONTRACTOR IS ALSO REQUIRED TO NOTIFY AND COORDINATE IRRIGATION CONTRACT WORK WITH ALL APPLICABLE CONTRACTORS FOR THE LOCATION AND INSTALLATION OF PIPE, CONDUIT OR SLEEVES THROUGH OR UNDER WALLS, ROADWAYS, PAVING, STRUCTURE, ETC., BEFORE CONSTRUCTION. IN THE EVENT THESE NOTIFICATIONS ARE NOT PERFORMED, THE CONTRACTOR SHALL ASSUME FULL RESPONSIBILITY FOR ALL REQUIRED REVISIONS.

2. THE CONTRACTOR SHALL EXERCISE CARE IN LOCATING PIPING AS TO NOT CONFLICT WITH OTHER UTILITIES. DO NOT INSTALL IRRIGATION PIPING PARALLEL TO AND DIRECTLY OVER OTHER UTILITIES.

3. THE INTENT OF THIS IRRIGATION SYSTEM IS TO PROVIDE THE MINIMUM AMOUNT OF WATER REQUIRED TO SUSTAIN GOOD PLANT HEALTH.

4. IT IS THE RESPONSIBILITY OF THE LANDSCAPE MAINTENANCE CONTRACTOR AND/OR OWNER TO PROGRAM THE IRRIGATION CONTROLLERS TO PROVIDE THE MINIMUM AMOUNT OF WATER NEEDED TO SUSTAIN GOOD PLANT HEALTH. THIS INCLUDES MAKING ADJUSTMENTS TO THE PROGRAM FOR SEASONAL WEATHER CHANGES, PLANT MATERIAL, WATER REQUIREMENTS, MOUNDS AND SLOPES, SUN, SHADE, AND WIND EXPOSURES.

5. AT THE END OF THE REQUIRED MAINTENANCE PERIOD OF THE CONTRACTOR, THE OWNER SHALL PROVIDE REGULAR MAINTENANCE OF THE IRRIGATION SYSTEM TO ENSURE THE EFFICIENT USE OF WATER. MAINTENANCE SHALL INCLUDE, BUT NOT BE LIMITED TO CHECKING, ADJUSTING, AND REPAIRING IRRIGATION EQUIPMENT AND CONTROL SYSTEM.

6. 120 VOLT A.C. (2.5 AMP DEMAND) ELECTRICAL SERVICE TO IRRIGATION CONTROLLER LOCATION TO BE PROVIDED UNDER ELECTRICAL CONTRACT WORK. IRRIGATION CONTRACTOR TO MAKE FINAL CONNECTION FROM ELECTRICAL STUB-OUT TO CONTROLLER AND PROVIDE PROPER GROUNDING PER CONTROLLER MANUFACTURER'S INSTRUCTIONS.

7. CONTROLLER SHALL HAVE ITS OWN GROUND ROD. THE GROUND ROD SHALL BE AN EIGHT FOOT LONG BY 5/8" DIAMETER U.L. APPROVED COPPER CLAD ROD. NO MORE THAN 6" OF THE GROUND ROD TO BE ABOVE GRADE. CONNECT #6 GAUGE WIRE WITH A U.L. APPROVED GROUND ROD CLAMP TO ROD AND BACK TO GROUND SCREW AT BASE OF CONTROLLER WITH APPROPRIATE CONNECTOR. THIS WIRE SHOULD BE AS SHORT AS POSSIBLE, AVOIDING ANY KINKS OR BENDING. GROUND ROD SHALL BE A MINIMUM OF EIGHT FEET (8') FROM IRRIGATION CONTROL WIRE BUNDLE.

8. CONTRACTOR SHALL PROGRAM THE IRRIGATION CONTROLLER TO PROVIDE IRRIGATION TO ALL PLANTING WITHIN THE ALLOWED WATERING WINDOW OF TIME AS REQUIRED. THE CONTRACTOR SHALL CREATE CONTROLLER PROGRAMING THAT WILL NOT EXCEED THE MAXIMUM GALLONS PER MINUTE FLOW RATE STATED ON THE DRAWINGS, AND NOT EXCEED THE CAPACITY OF ANY MAIN LINE PIPING.

9. IRRIGATION CONTROL WIRES SHALL BE HUNTER JACKETED DECODER CABLES WITH U.L. APPROVAL FOR DIRECT BURIAL IN GROUND, SIZE #14-1. SPLICE SHALL BE MADE WITH 3M-DBR/Y-6 SEAL PACKS.

10. FLOW SENSOR CABLE SHALL BE A SHIELDED, TWISTED PAIR CABLE, SIZE #16. NO SPLICES ALLOWED.

11. SPlicing OF DECODER CABLES IS NOT PERMITTED EXCEPT IN VALVE BOXES. SEAL WIRE SPLICES WITH 3M-DBR/Y-6 SPLICE SEALING DEVICES OF SIZE COMPATIBLE WITH WIRE SIZE. LEAVE A 36" LONG, 1" DIAMETER COIL OF EXCESS WIRE AT EACH SPLICE AND A 36" LONG EXPANSION LOOP EVERY 100 FEET ALONG WIRE RUN. TAPE WIRES TOGETHER EVERY TEN FEET. TAPING WIRES IS NOT REQUIRED INSIDE SLEEVES.

12. PLASTIC VALVE BOXES ARE TO BE BLACK IN COLOR WITH BOLT DOWN, NON-HINGED COVER MARKED "IRRIGATION". BOX BODY SHALL HAVE KNOCK OUTS. MANUFACTURER SHALL BE CARSON INDUSTRIES.

13. INSTALL REMOTE CONTROL VALVE BOXES 12" FROM WALK, CURB, LAWN, HEADER BOARD, BUILDING, OR LANDSCAPE FEATURE. AT MULTIPLE VALVE BOX GROUPS, EACH BOX SHALL BE AN EQUAL DISTANCE FROM THE WALK, CURB, LAWN, ETC. AND EACH BOX SHALL BE 12" APART. SHORT SIDE OF RECTANGULAR VALVE BOXES SHALL BE PARALLEL TO WALK, CURB, ETC.

14. VALVE LOCATIONS SHOWN ARE DIAGRAMMATIC. INSTALL IN GROUND COVER/SHRUB AREAS WHERE POSSIBLE (NOT IN LAWN AREA).

15. THE IRRIGATION CONTRACTOR SHALL FLUSH AND ADJUST ALL SPRINKLER HEADS FOR OPTIMUM PERFORMANCE AND TO PREVENT OVER SPRAY ONTO WALKS, ROADWAYS, AND/OR BUILDINGS AS MUCH AS POSSIBLE. THIS SHALL INCLUDE SELECTING THE BEST DEGREE OF ARC TO FIT THE EXISTING SITE CONDITIONS AND TO THROTTLE THE FLOW CONTROL AT EACH VALVE TO OBTAIN THE OPTIMUM OPERATING PRESSURE FOR EACH SYSTEM.

16. ALL SPRINKLER HEADS SHALL BE SET PERPENDICULAR TO FINISH GRADE OF THE AREA TO BE IRRIGATED UNLESS OTHERWISE NOTED ON THE DRAWINGS.

17. LOCATE BUBBLERS ON UP-HILL SIDE OF PLANT OR TREE.

18. INSTALL A VALCON 5000 SERIES SPRING LOADED CHECK VALVE BELOW THOSE SPRINKLERS WHERE LOW HEAD DRAINAGE WILL CAUSE EROSION AND/OR EXCESS WATER.

19. WHERE IT IS NECESSARY TO EXCAVATE ADJACENT TO EXISTING TREES, THE CONTRACTOR SHALL USE ALL POSSIBLE CARE TO AVOID INJURY TO TREES AND TREE ROOTS. EXCAVATION IN AREAS WHERE TWO (2) INCH AND LARGER ROOTS OCCUR SHALL BE DONE BY HAND. TRENCHES ADJACENT TO TREE SHOULD BE CLOSED WITHIN TWENTY-FOUR (24) HOURS; AND WHERE THIS IS NOT POSSIBLE, THE SIDE OF THE TRENCH ADJACENT TO THE TREE SHALL BE KEPT SHADED WITH BURLAP OR CANVAS.

20. IRRIGATION CONTRACTOR TO NOTIFY ALL LOCAL JURISDICTIONS FOR INSPECTION AND TESTING OF INSTALLED BACKFLOW PREVENTION DEVICE.

21. PRESSURE TEST PROCEDURE. THE CONTRACTOR SHALL:

A. NOTIFY ARCHITECT AT LEAST THREE (3) DAY IN ADVANCE OF TESTING.

B. PERFORM TESTING AT HIS OWN EXPENSE.

C. CENTER LOAD PIPING WITH SMALL AMOUNT OF BACKFILL TO PREVENT ARCHING OR SLIPPING UNDER PRESSURE. NO FITTING SHALL BE COVERED.

D. APPLY THE FOLLOWING TESTS AFTER WELD PLASTIC PIPE JOINTS HAVE CURED AT LEAST 24 HOURS.

1. TEST LIVE (CONSTANT PRESSURE) AND QUICK COUPLER LINE HYDROSTATICALLY AT 125 PSI. MINIMUM. LINES WILL BE APPROVED IF TEST PRESSURE IS MAINTAINED FOR SIX (6) HOURS. THE LINE WILL BE APPROVED OR NOT APPROVED AS SUCH RESULTS MAY INDICATE. THE CONTRACTOR SHALL MAKE TESTS AND REPAIRS AS NECESSARY UNTIL TEST CONDITIONS ARE MET.

2. TEST RCV CONTROLLED LATERAL LINES WITH WATER AT LINE PRESSURE AND VISUALLY INSPECT FOR LEAKS. RETEST AFTER CORRECTING DEFECTS.

22. THE SPRINKLER SYSTEM DESIGN IS BASED ON THE MINIMUM OPERATING PRESSURE SHOWN ON THE IRRIGATION DRAWINGS. THE IRRIGATION CONTRACTOR SHALL VERIFY WATER PRESSURE PRIOR TO CONSTRUCTION. REPORT ANY DIFFERENCE BETWEEN THE WATER PRESSURE INDICATED ON THE DRAWINGS AND THE ACTUAL PRESSURE READING AT THE IRRIGATION POINT OF CONNECTION TO THE OWNER'S AUTHORIZED REPRESENTATIVE.

23. IRRIGATION DEMAND: 19 GPM AT 75 PSI STATIC PRESSURE AT IRRIGATION POINT OF CONNECTION. FIELD VERIFY WATER PRESSURE PRIOR TO CONSTRUCTION. IF ACTUAL WATER PRESSURE DIFFERS FROM THE STATED PRESSURE CONTACT ARCHITECT FOR DIRECTION AND POSSIBLE REVISION.

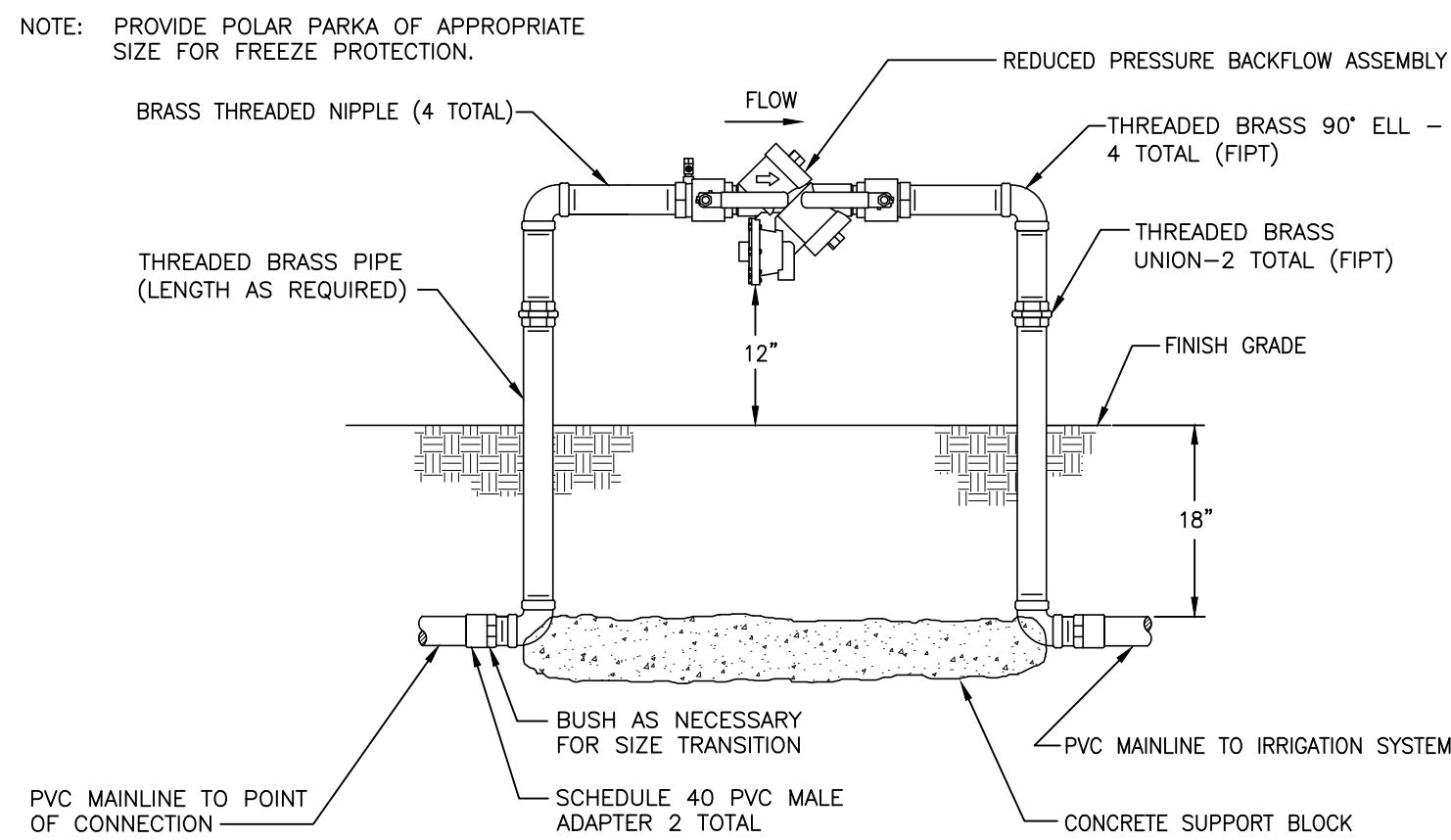
24. PIPE THREAD SEALANT COMPOUND SHALL BE RECTOR SEAL T+2, CHRISTY'S ULTRA SEAL, OR APPROVED EQUAL.

25. RECORD DRAWINGS:

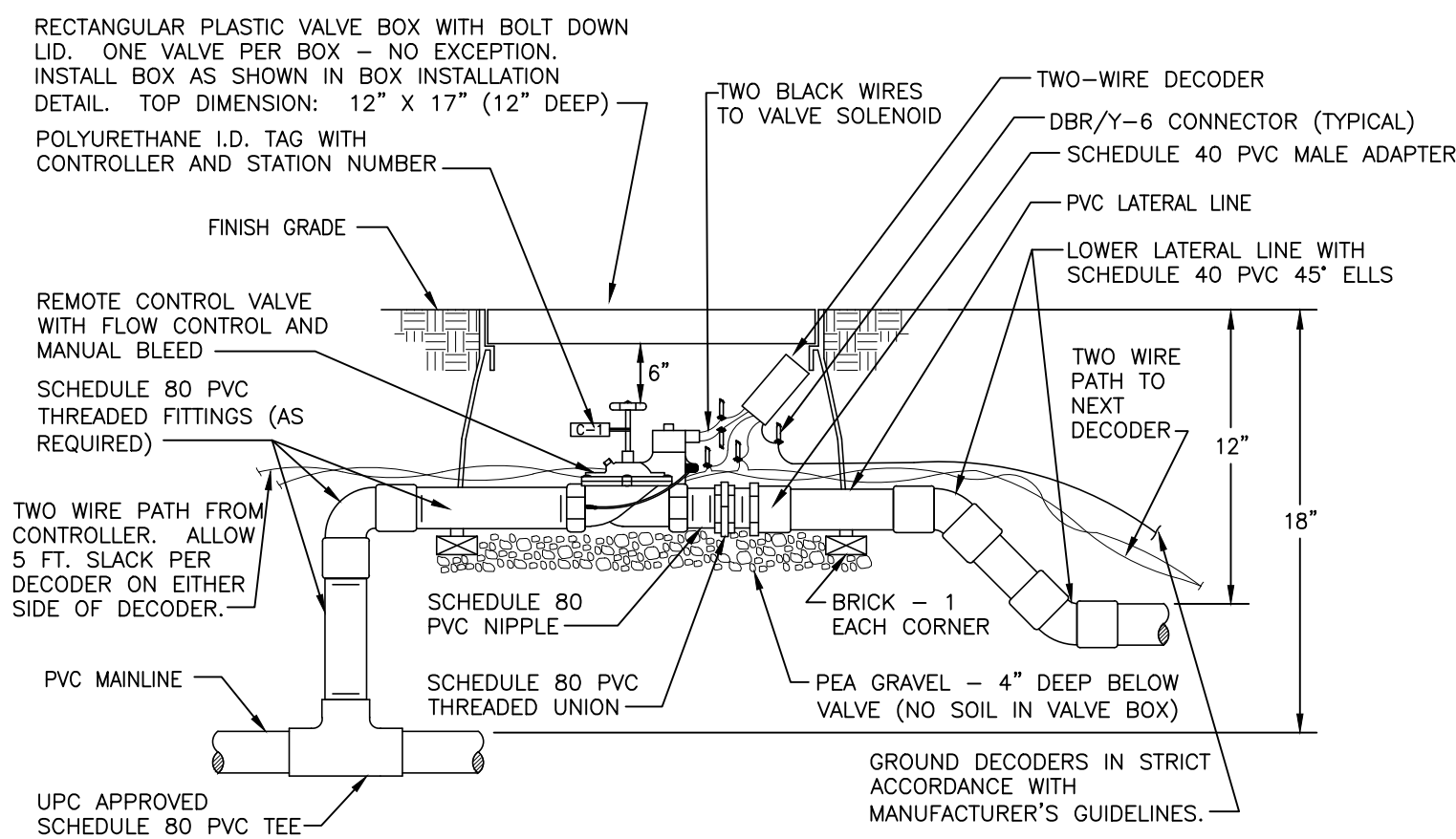
A. THE CONTRACTOR SHALL MAINTAIN IN GOOD ORDER IN THE FIELD OFFICE ONE COMPLETE SET OF BLACK LINE PRINTS OF ALL SPRINKLER DRAWINGS WHICH FORM A PART OF THE CONTRACT, SHOWING ALL WATER LINES, SPRINKLERS, VALVES, CONTROLLERS AND STUB-OUTS. IN THE EVENT ANY WORK IS NOT INSTALLED AS INDICATED ON THE DRAWINGS, SUCH WORK SHALL BE CORRECTED AND DIMENSIONED ACCURATELY FROM THE BUILDING WALLS.

B. ALL UNDERGROUND STUB-OUTS FOR FUTURE CONNECTIONS AND VALVES SHALL BE LOCATED AND DIMENSIONED ACCURATELY FROM BUILDING WALLS ON ALL RECORD DRAWINGS.

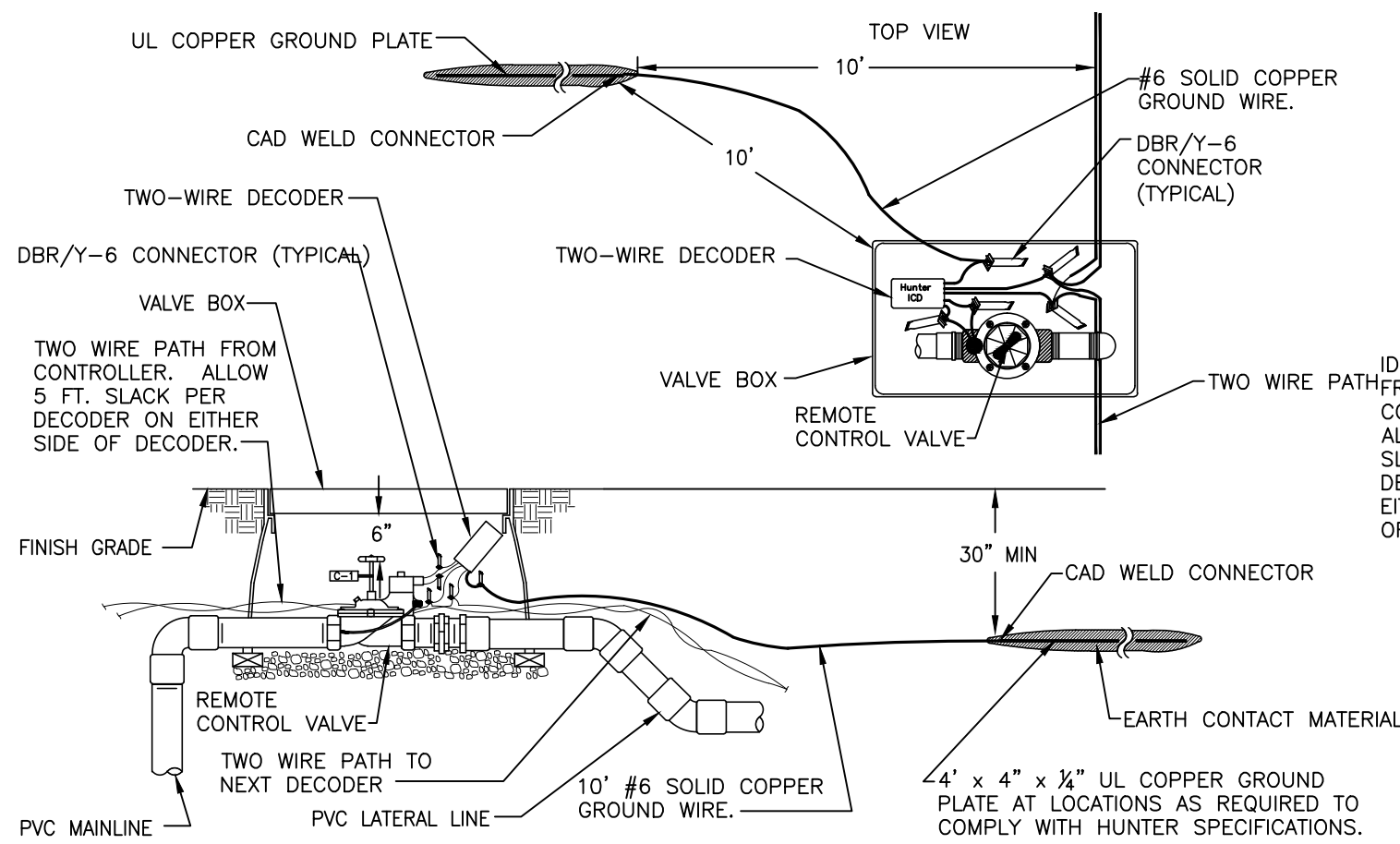
C. UPON COMPLETION OF THE WORK, OBTAIN REPRODUCIBLE PRINTS FROM ARCHITECT AND NEATLY CORRECT THE PRINTS TO SHOW THE AS-BUILT CONDITIONS.
- IRRIGATION LEGEND
- | SYMBOL | MODEL NUMBER | DESCRIPTION |
|--------|--|--|
| | PROS-12-PRS40-CV-MP3000360 | HUNTER POP-UP SPRAY SPRINKLER WITH MP ROTATOR NOZZLE (SHRUB) |
| | PROS-12-PRS40-CV-MP3000090 | HUNTER POP-UP SPRAY SPRINKLER WITH MP ROTATOR NOZZLE (SHRUB) |
| | PROS-12-PRS40-CV-MP3000090 | HUNTER POP-UP SPRAY SPRINKLER WITH MP ROTATOR NOZZLE (SHRUB) |
| | PROS-12-PRS40-CV-MP2000360 | HUNTER POP-UP SPRAY SPRINKLER WITH MP ROTATOR NOZZLE (SHRUB) |
| | PROS-12-PRS40-CV-MP2000090 | HUNTER POP-UP SPRAY SPRINKLER WITH MP ROTATOR NOZZLE (SHRUB) |
| | PROS-12-PRS40-CV-MP2000090 | HUNTER POP-UP SPRAY SPRINKLER WITH MP ROTATOR NOZZLE (SHRUB) |
| | PROS-12-PRS40-CV-MP1000090 | HUNTER POP-UP SPRAY SPRINKLER WITH MP ROTATOR NOZZLE (SHRUB) |
| | PROS-12-PRS40-CV-MP1000090 | HUNTER POP-UP SPRAY SPRINKLER WITH MP ROTATOR NOZZLE (SHRUB) |
| | PROS-12-PRS30-CV-15-H,Q | HUNTER POP-UP SPRAY SPRINKLER (SHRUB) |
| | PROS-12-PRS30-CV-12-F,H,Q | HUNTER POP-UP SPRAY SPRINKLER (SHRUB) |
| | PROS-12-PRS30-CV-10-F,H,Q | HUNTER POP-UP SPRAY SPRINKLER (SHRUB) |
| | PROS-12-PRS30-CV-8-H,Q | HUNTER POP-UP SPRAY SPRINKLER (SHRUB) |
| | PROS-12-PRS30-CV-5-H,Q | HUNTER POP-UP SPRAY SPRINKLER (SHRUB) |
| | M210 | BOWSMITH MULTI-OUTLET EMITTER (1 GPH PER OUTLET) |
| | PCB-25 | HUNTER BUBBLER (SHRUB) |
| | PCB-25 | HUNTER BUBBLER (TREE) |
| | RZWS-36-25-CV | HUNTER ROOT ZONE BUBBLER ASSEMBLY AND CHECK VALVE. (TREE) |
| | 1" COMPACT & 90-1" | AMIAD FILTER W/155 MESH SCREEN AND WILKINS PRESSURE REDUCING VALVE |
| | 3200-1½" | SUPERIOR NORMALLY CLOSED MASTER CONTROL VALVE |
| | FSI-T10-001-1"/P7162D-A | CREATIVE SENSOR TECHNOLOGY FLOW SENSOR WITH PAIGE SHIELDED COMMUNICATION CABLE |
| | ICV-FS-SERIES | HUNTER FILTER SENTRY REMOTE CONTROL VALVE |
| | T-113 | NIBCO GATE VALVE (LINE SIZE) |
| | I-401 | CHAMPION HOSE BIB |
| | 975XL2-1" | WILKINS REDUCED PRESSURE BACKFLOW ASSEMBLY (LEAD FREE) |
| | 182199L | GROUNDING PLATE LOCATION FOR GROUNDING IRRIGATION CONTROL TWO-WIRE PATH. SEE DETAILS AND MANUFACTURER'S (PAIGE ELECTRICAL COMPANY) INSTRUCTIONS |
| | ACC-99D-PED-SS/ICD-XXX/ICD-HP/ROAMXL-KIT | HUNTER ACC TWO WIRE DECODER CONTROLLER, ROAMXL REMOTE KIT, STATION DECODERS, DECODER PROGRAMMER, AND ALL EARTH GROUNDING EQUIPMENT. CONTACT DON FRANKLIN, HUNTER INDUSTRIES, (916-899-9437) WITH QUESTIONS REGARDING TWO WIRE CONTROLLER COMPONENTS, INSTALLATION, WIRING, WIRE SPLICING, GROUNDING AND DECODER PROGRAMMING. (PEDESTAL MOUNT IN STAINLESS STEEL ENCLOSURE) |
| | SOLAR SYNC-SEN | HUNTER SOLAR SYNC SENSOR |
| | | PRECIPITATION RATE |
| | | CONTROLLER & STATION NUMBER |
| | | APPROXIMATE FLOW (GPM) |
| | | REMOTE CONTROL VALVE SIZE |
| | | MAIN LINE: 1120-SCHEDULE 40 PVC PLASTIC PIPE WITH SCHEDULE 40 PVC SOLVENT-WELD FITTINGS. 18" COVER. |
| | | LATERAL LINE: 1120-SCHEDULE 40 PVC PLASTIC PIPE WITH SCHEDULE 40 PVC SOLVENT-WELD FITTINGS. 12" COVER. |
| | | SLEEVING: 1120-SCHEDULE 40 PVC PLASTIC PIPE. 18" COVER. 24" UNDER VEHICULAR PAVING. |
- DICKSON & ASSOCIATES, INC.
- LANDSCAPE IRRIGATION
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- Consultant:
- Stamp:
- Project Name:
- Rancho
Arroyo
Claro
- 80 Clear Creek Rd.
Napa, CA
- Sheet Title:
- Irrigation Legend,
Notes, Water Use
Calculations
- | | |
|-----------------------|----------|
| Submittal: | Date: |
| Conformed Pricing Set | 11/11/13 |
| Permit Set | 12/09/13 |
| 100% CD Set | 01/31/14 |
| 100% CD Planting Set | 02/07/14 |
| 100% CD Set - REV3 | 02/20/14 |
- Revision: Date:
- Scale:
- Sheet:
- L7.3
- RANCHO ARROYO CLARO/A/00B/14-702/L7.3_Irrigation Plan/ L7.3 /SYMBOL SIZE: NONE / 03-05-2014 CK



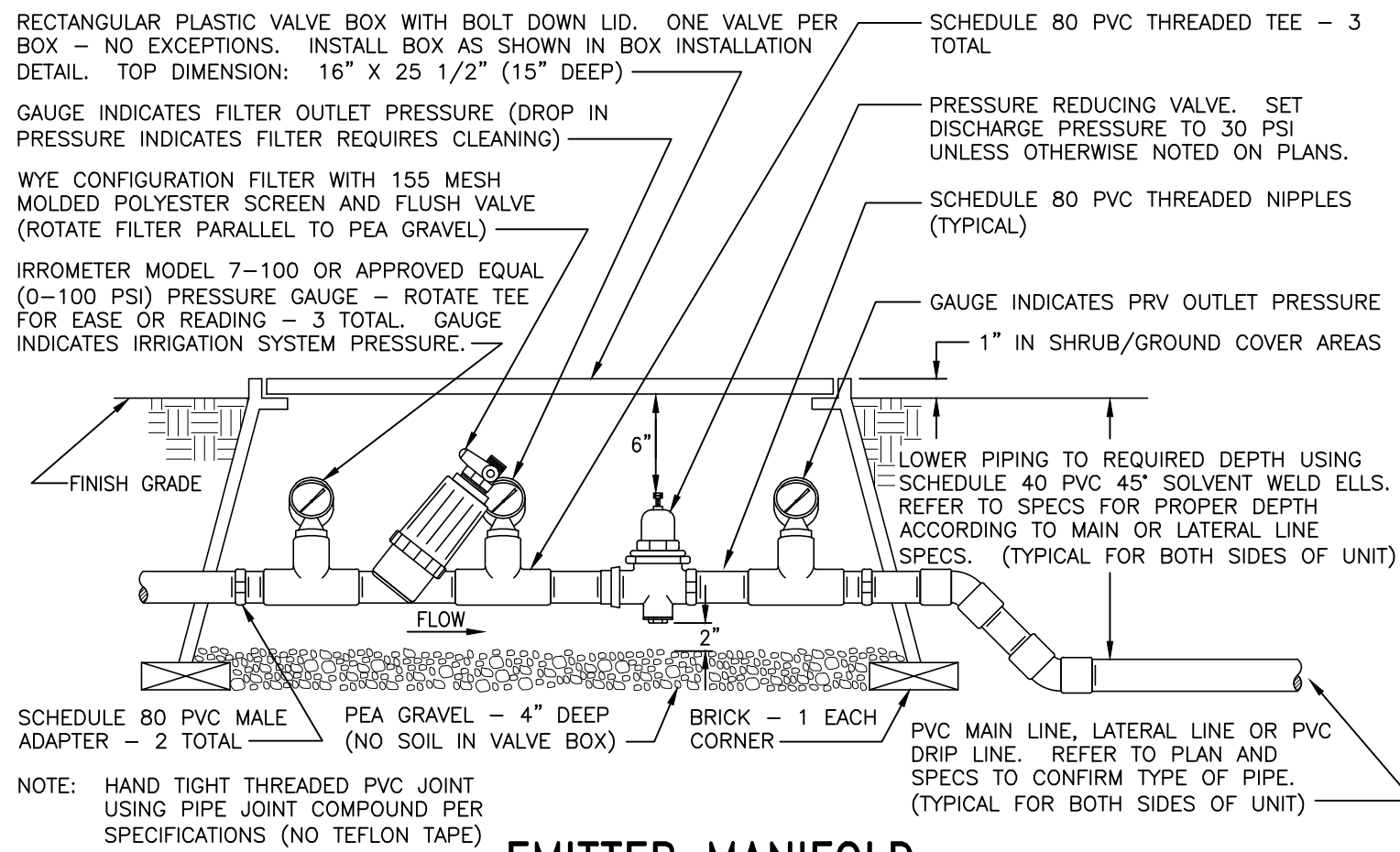
REDUCED PRESSURE BACKFLOW ASSEMBLY
NOT TO SCALE



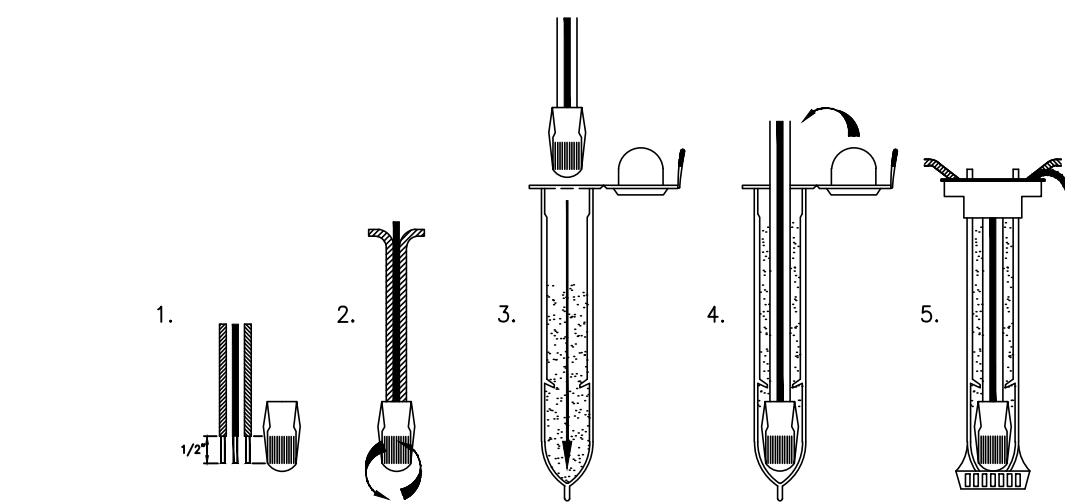
REMOTE CONTROL VALVE
NOT TO SCALE



DECODER GROUNDING
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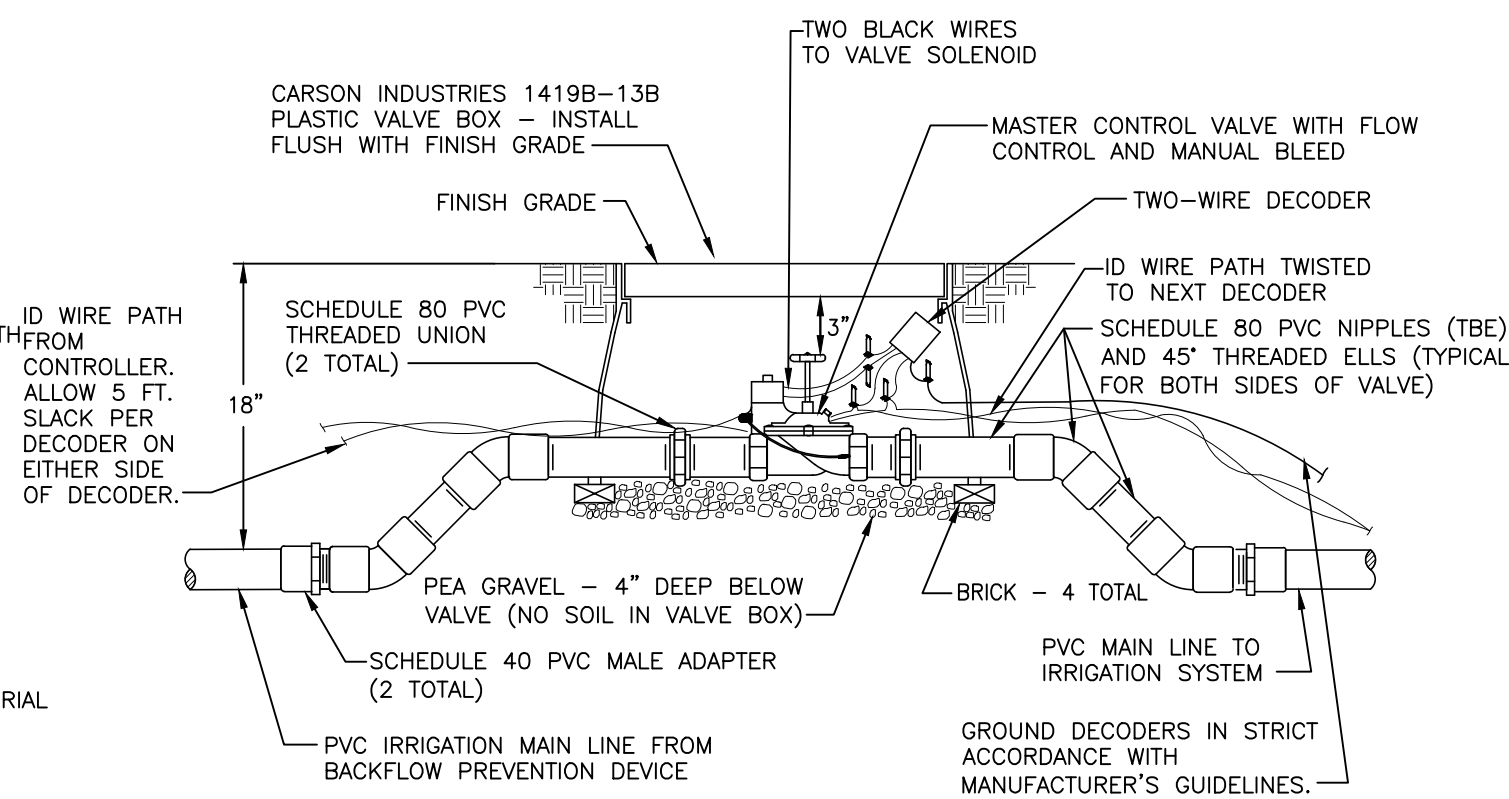


EMITTER MANIFOLD
NOT TO SCALE

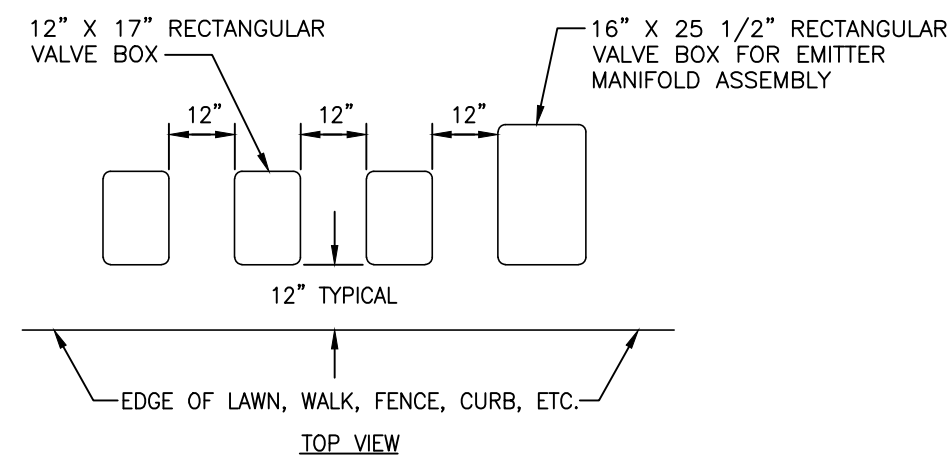


- INSTRUCTIONS:
1. USE 3M-DBR/Y-6 WEATHER PROOF SPLICE.
 2. STRIP WIRES APPROXIMATELY 1/2" (12.7 MM) TO EXPOSE WIRE.
 3. TWIST CONNECTOR AROUND WIRES CLOCKWISE UNTIL HAND TIGHT, DO NOT OVERTIGHTEN.
 4. INSERT WIRE ASSEMBLY INTO PLASTIC TUBE UNTIL WIRE CONNECTOR SNAPS PAST LIP IN BOTTOM OF TUBE.
 5. PLACE WIRES WHICH EXIT TUBE IN WIRE EXIT HOLES AND CLOSE CAP UNTIL IT SNAPS.
 6. INSPECT FINAL SPLICE ASSEMBLY TO BE SECURE AND FINISHED.

WEATHERPROOF SPLICE ASSEMBLY
NOT TO SCALE

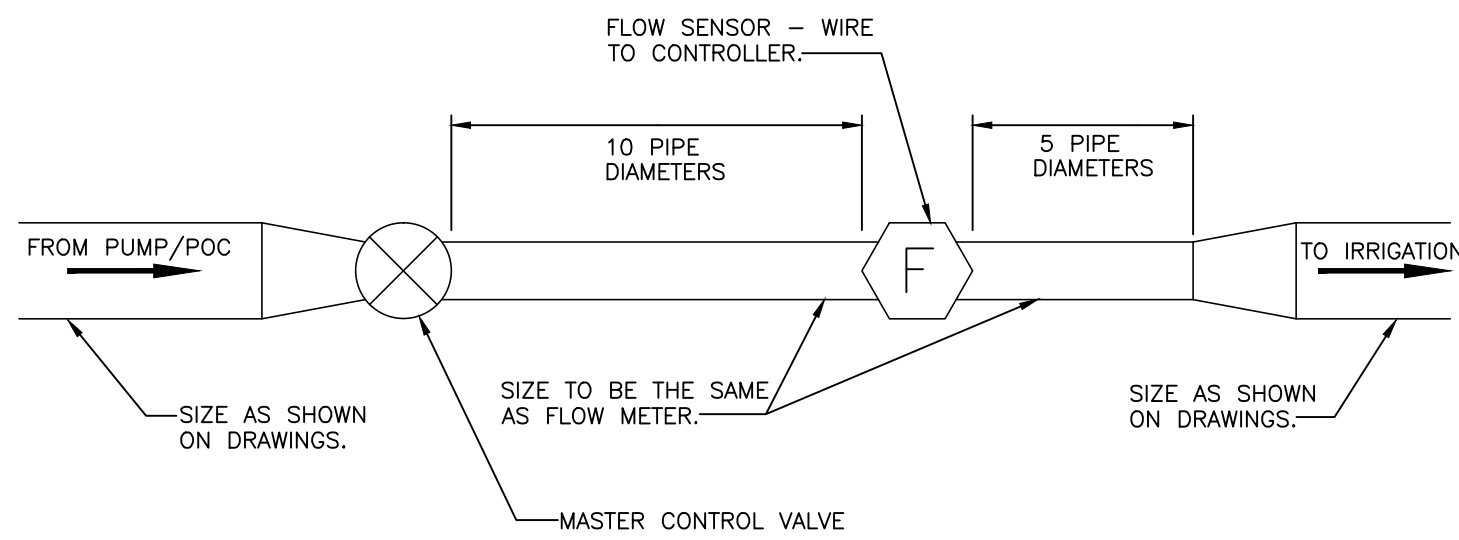


MASTER CONTROL VALVE
NOT TO SCALE

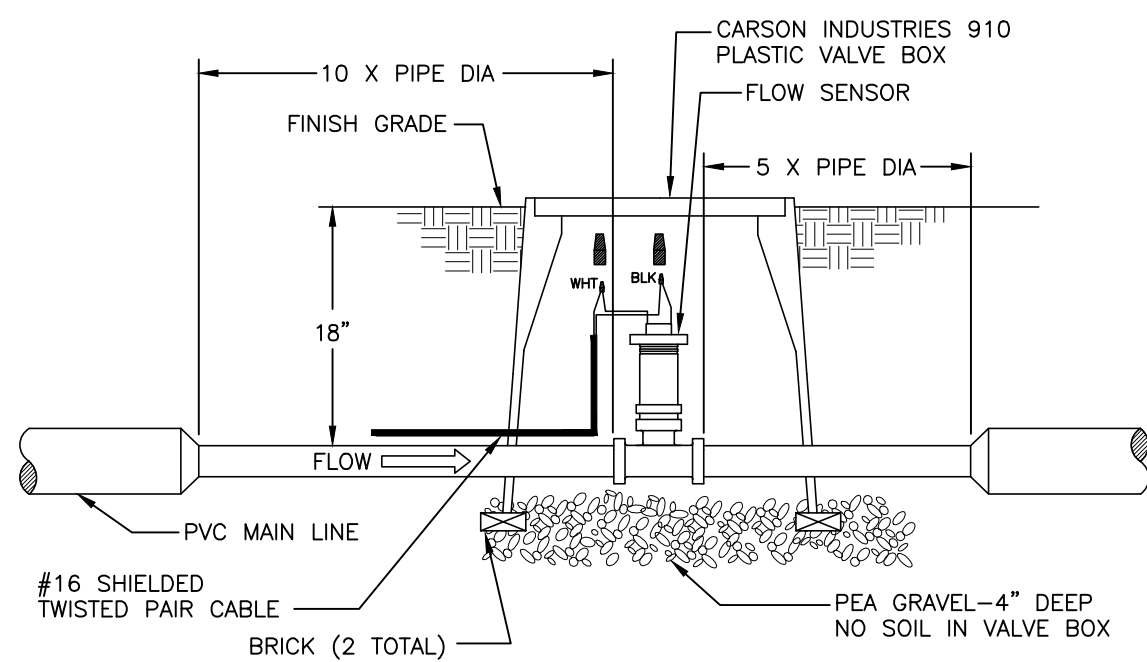


1. CENTER VALVE BOX OVER REMOTE CONTROL VALVE TO FACILITATE SERVICING VALVE.
2. SET BOXES 1" ABOVE FINISH GRADE OR MULCH COVER IN GROUND COVER/SHRUB AREA AND FINISH GRADE IN TURF AREA.
3. SET RCV AND VALVE BOX ASSEMBLY IN GROUND COVER/SHRUB AREA WHERE POSSIBLE. INSTALL IN LAWN ONLY IF GROUND COVER DOES NOT EXIST ADJACENT TO LAWN.
4. SET BOXES PARALLEL TO EACH OTHER AND PERPENDICULAR TO EDGE OF LAWN, WALK, FENCE, CURB, ETC.
5. AVOID HEAVILY COMPACTING SOIL AROUND VALVE BOXES TO PREVENT COLLAPSE AND DEFORMATION OF VALVE BOX SIDES.
6. INSTALL EXTENSION BY VALVE BOX MANUFACTURER AS REQUIRED TO COMPLETELY ENCLOSE ASSEMBLY FOR EASY ACCESS.

VALVE BOX INSTALLATION
NOT TO SCALE



MASTER VALVE / FLOW SENSOR
NOT TO SCALE



FLOW SENSOR INSTALLATION
NOT TO SCALE

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LANDSCAPE IRRIGATION

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Consultant:

Stamp:

Project Name:

Rancho Arroyo Claro

**80 Clear Creek Rd.
Napa, CA**

Sheet Title:

**Irrigation
Details**

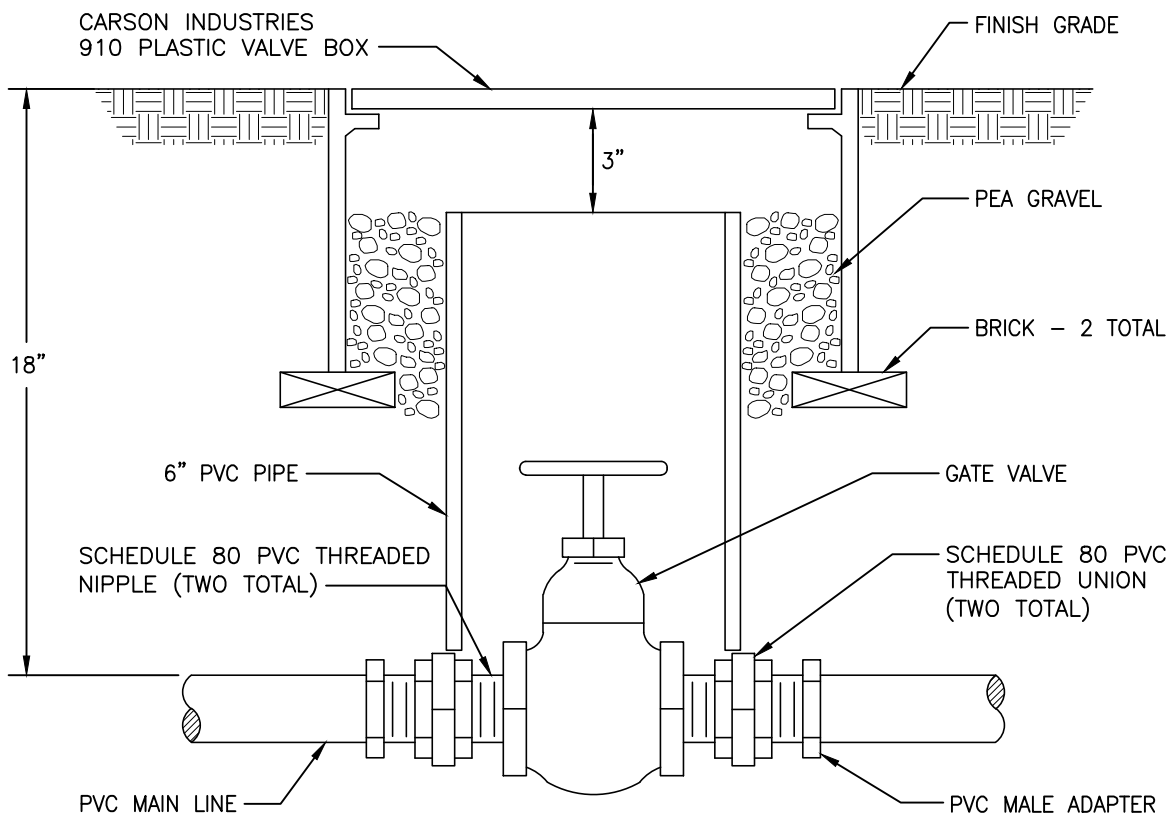
Submittal:	Date:
Conformed Pricing Set	11/11/13
Permit Set	12/09/13
100% CD Set	01/31/14
100% CD Planting Set	02/07/14
100% CD Set - REV3	02/20/14

Revision: Date:

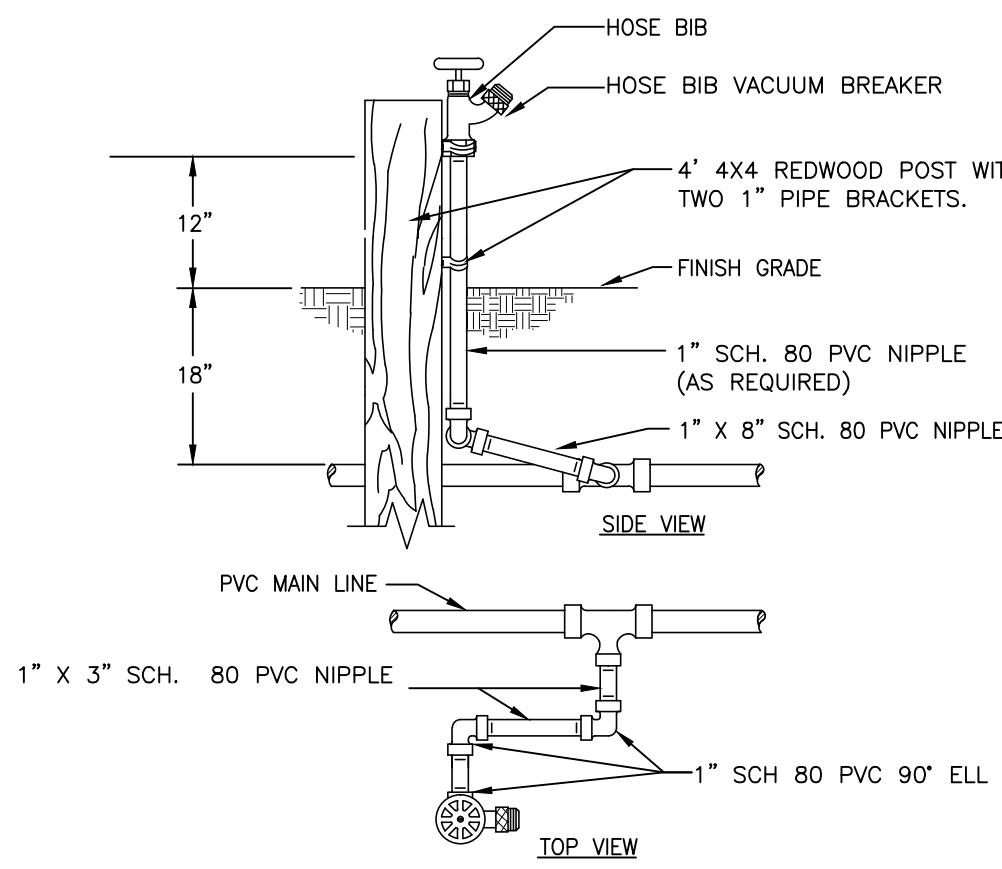
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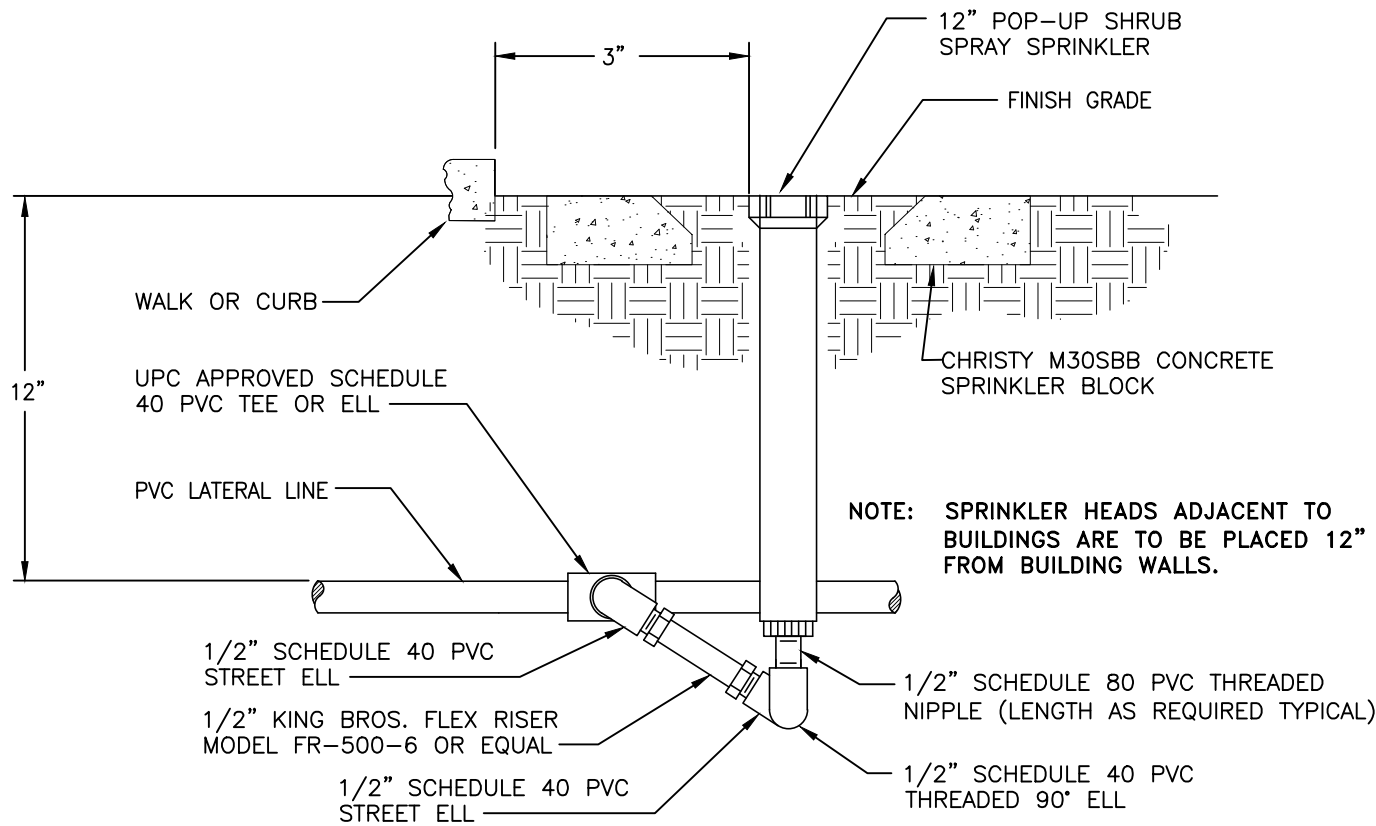
L7.4



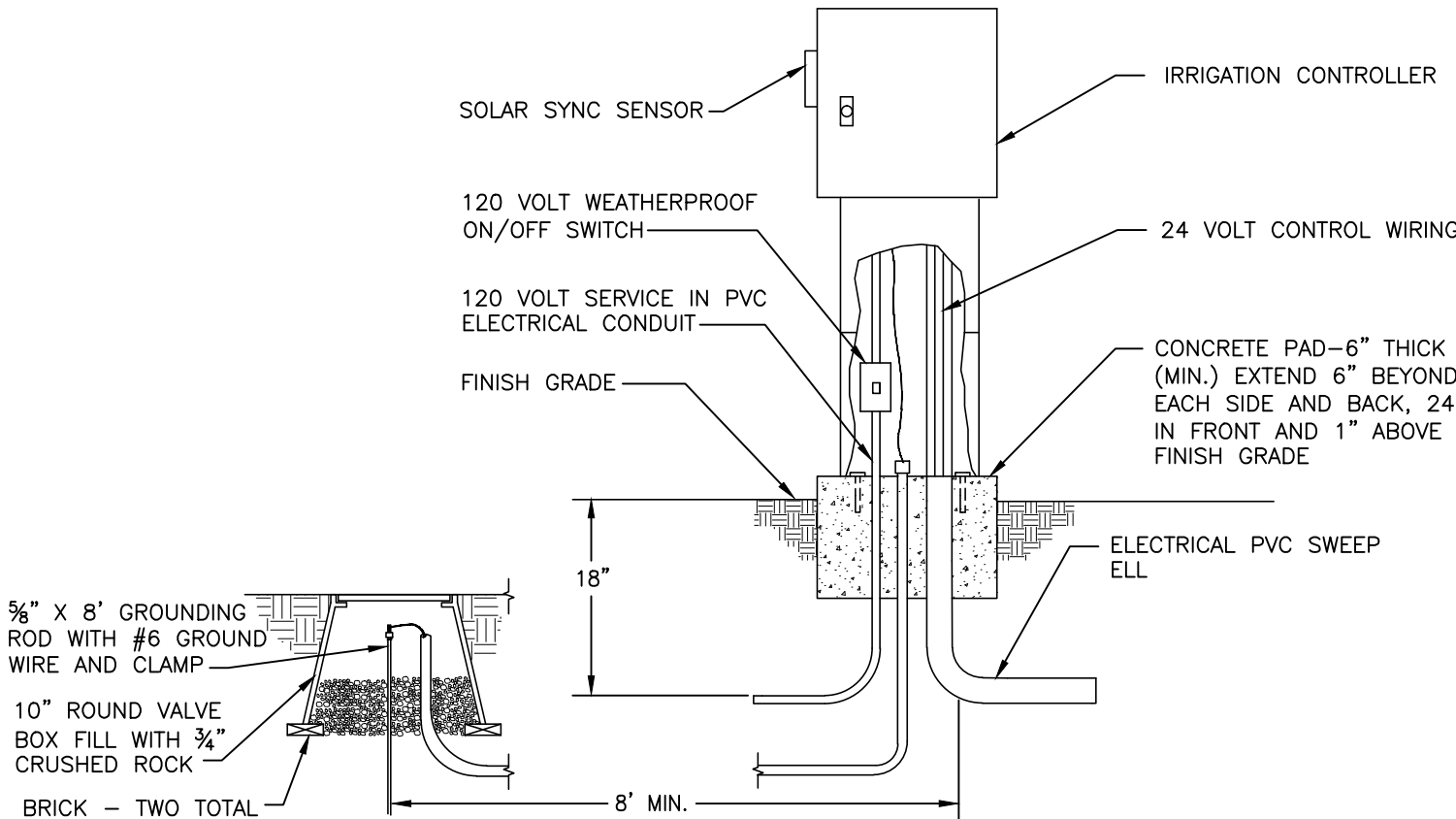
GATE VALVE
NOT TO SCALE



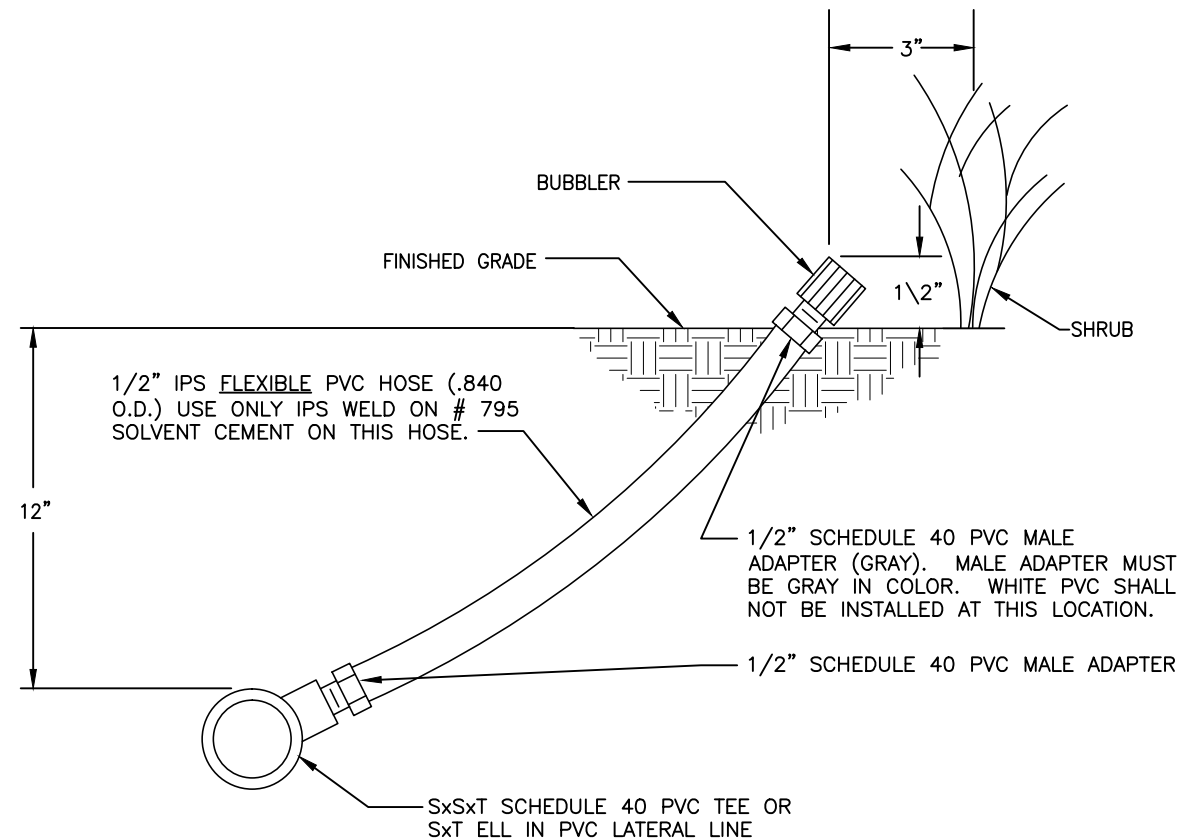
HOSE BIB
NOT TO SCALE



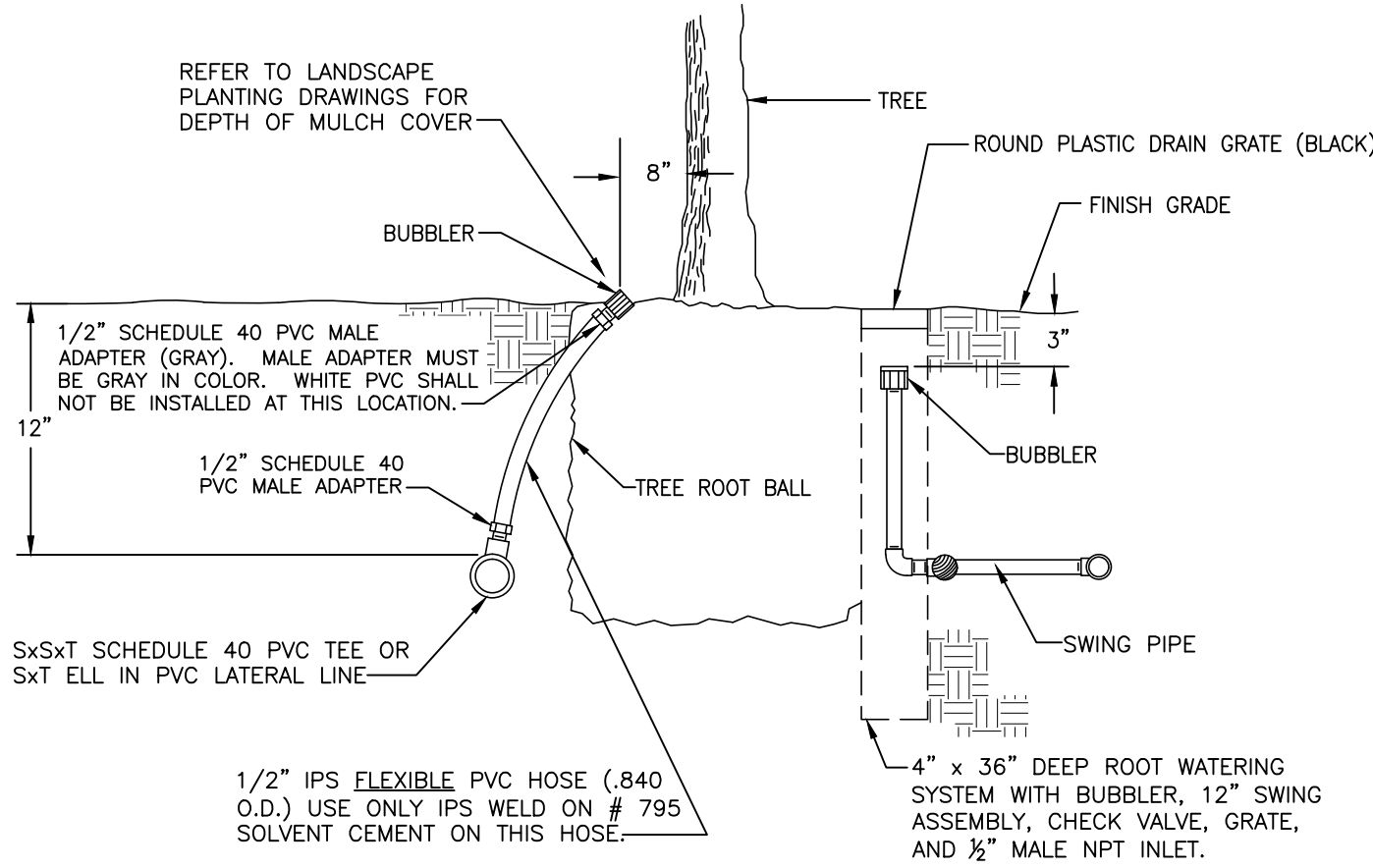
POP-UP SPRAY SPRINKLER RISER
NOT TO SCALE



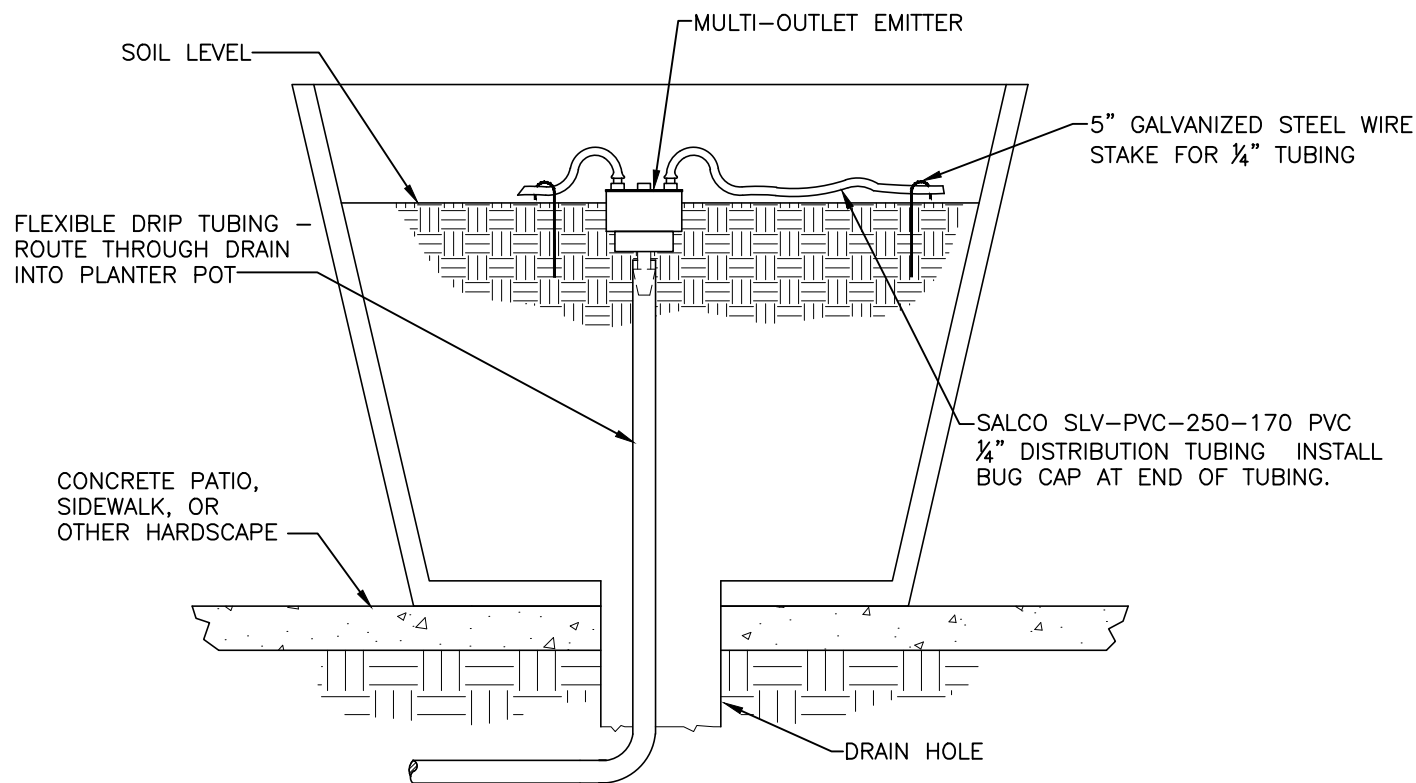
PEDESTAL MOUNT CONTROLLER
NOT TO SCALE



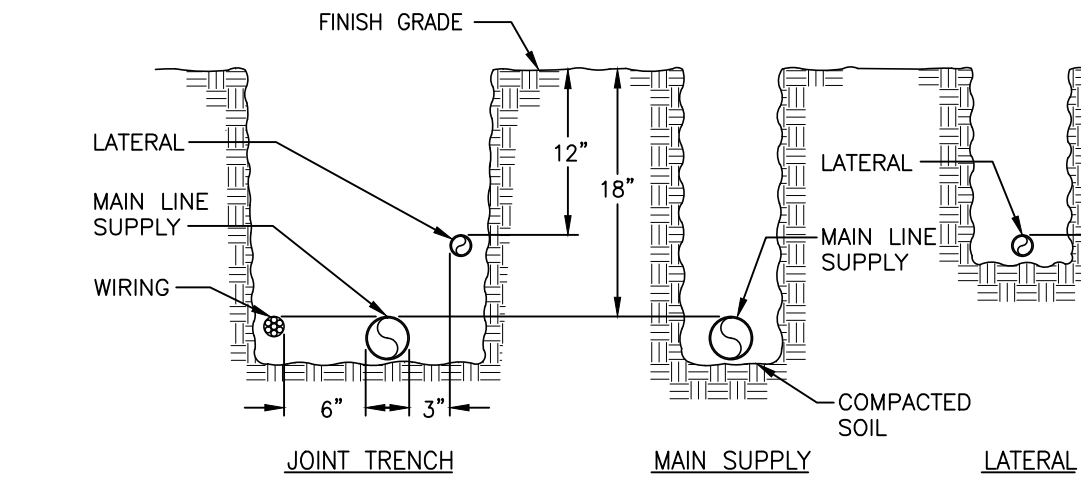
SHRUB BUBBLER WITH FLEXIBLE RISER
NOT TO SCALE



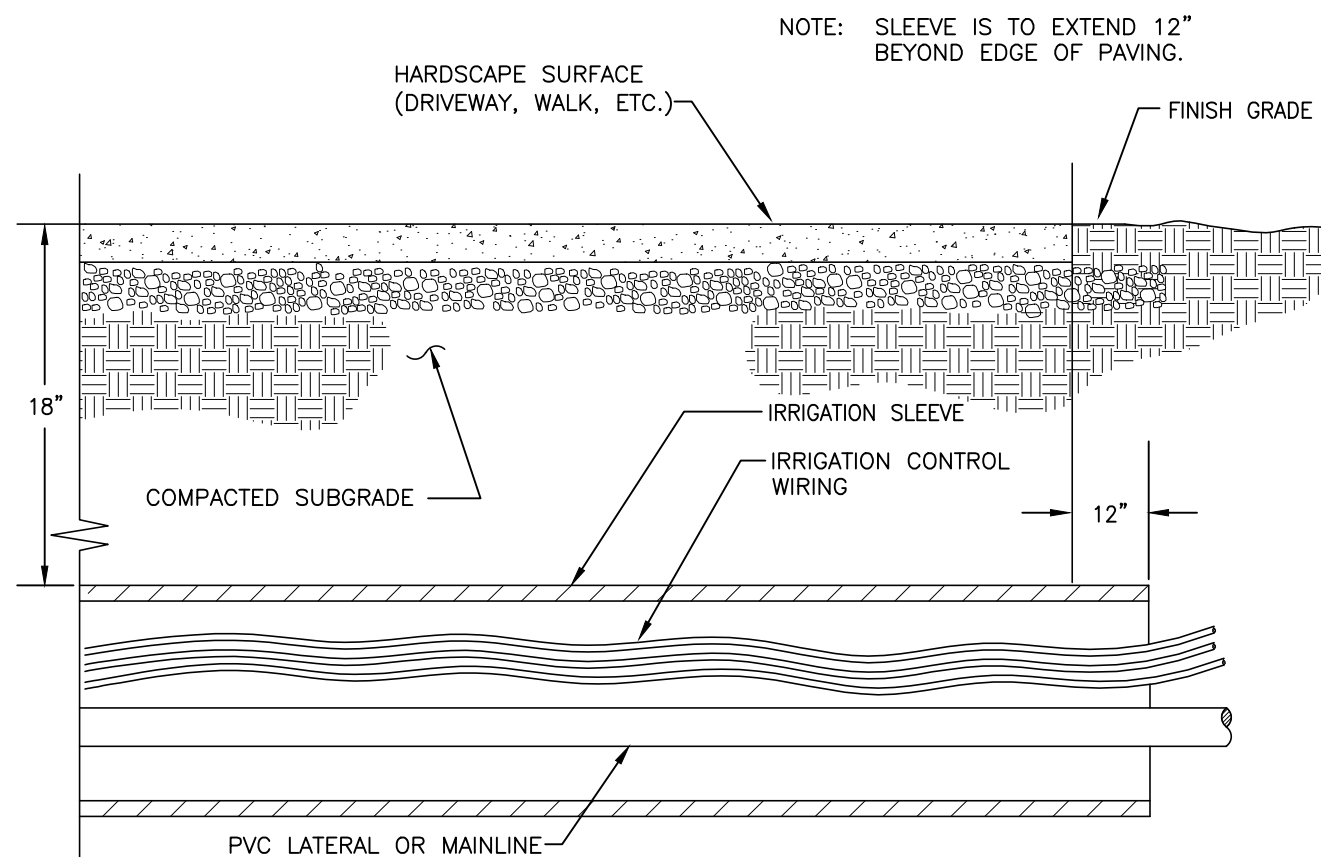
TREE BUBBLER
NOT TO SCALE



PLANTER POT IRRIGATION
NOT TO SCALE



TRENCHING DETAIL
NOT TO SCALE



SLEEVING INSTALLATION
NOT TO SCALE

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Consultant:

Stamp:

Project Name:

**Rancho
Arroyo
Claro**

**80 Clear Creek Rd.
Napa, CA**

Sheet Title:

**Irrigation
Details**

Submittal:

Conformed Pricing Set 11/11/13

Permit Set 12/09/13

100% CD Set 01/31/14

100% CD Planting Set 02/07/14

100% CD Set - REV3 02/20/14

Revision:

Date:

Scale:

Sheet:

L7.5

DICKSON & ASSOCIATES, INC.
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Napa, CA

Sheet Title:
Irrigation
Schedules

Submittal: Date:
Conformed Pricing Set 11/11/13
Permit Set 12/09/13
100% CD Set 01/31/14
100% CD Planting Set 02/07/14
100% CD Set - REV3 02/20/14

Revision: Date:

Scale:

Sheet:

L7.6

IRRIGATION WATERING SCHEDULES

MULTI-OUT DRIP IRRIGATION FOR MODERATE WATER USE PLANTS IN POTS – STATIONS: 48, & 56															
SPRINKLER MANUFACTURER:		BOWSMITH		LOCATION:		NAPA, CALIFORNIA									
PRECIPITATION RATE (INCHES/HOUR):		0.57		EMITTER TUBE SPACING:		VARIES									
IRRIGATION SYSTEM EFFICIENCY		0.90		EMITTER TUBE FLOW:		1 GPH									
PLANT FACTOR:		0.50													
YEAR 2 REDUCTION AMOUNT:		-10% OF YEAR 1 (ESTABLISHMENT) RUN TIME MINUTES													
MONTH:		JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC	TOTAL	
ETO PER MONTH (INCHES):		1.30	1.70	2.80	3.90	5.10	6.00	7.10	6.10	4.80	3.10	1.50	0.90	44.30	
ETO PER WEEK (INCHES):		0.300	0.393	0.647	0.901	1.178	1.386	1.640	1.409	1.109	0.716	0.346	0.208		
APPLIED ETO PER WEEK (INCHES):		0.167	0.218	0.359	0.500	0.654	0.770	0.911	0.783	0.616	0.398	0.192	0.115		
MINUTES OF WATER PER WEEK:	YEAR 1	18	23	38	53	69	81	96	82	65	42	20	12		
	YEAR 2	16	21	34	47	62	73	86	74	58	38	18	11		
DAYS PER WEEK:	YEAR 1	1	1	2	3	4	5	5	4	3	2	1			
	YEAR 2	1	1	2	3	4	5	5	4	3	2	1			
MINUTES OF WATER PER DAY:	YEAR 1	18	23	19	18	17	16	19	16	16	14	10	12		
	YEAR 2	16	21	17	16	15	15	17	15	15	13	9	11		
CYCLES PER DAY:	YEAR 1	1	1	1	1	1	1	1	1	1	1	1	1		
	YEAR 2	1	1	1	1	1	1	1	1	1	1	1	1		
MINUTES PER CYCLE:	YEAR 1	18	23	19	18	17	16	19	16	16	14	10	12		
	YEAR 2	16	21	17	16	15	15	17	15	15	13	9	11		

BUBBLER IRRIGATION FOR LOW WATER USE SHRUB/GROUNDCOVER – STATIONS: 2, 4, 9, 19, 21, 33, 44, 55, 61, 65, 67, & 68															
SPRINKLER MANUFACTURER:		HUNTER		LOCATION:		NAPA, CALIFORNIA									
PRECIPITATION RATE (INCHES/HOUR):		4.85		HEAD SPACING:		VARIES									
IRRIGATION SYSTEM EFFICIENCY		0.85		HEAD GPM:		0.25									
PLANT FACTOR:		0.30													
YEAR 2 REDUCTION AMOUNT:		-10% OF YEAR 1 (ESTABLISHMENT) RUN TIME MINUTES													
		MONTH:	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC	TOTAL
		ETO PER MONTH (INCHES):	1.30	1.70	2.80	3.90	5.10	6.00	7.10	6.10	4.80	3.10	1.50	0.90	44.30
		ETO PER WEEK (INCHES):	0.300	0.393	0.647	0.901	1.178	1.386	1.640	1.409	1.109	0.716	0.346	0.208	
		APPLIED ETO PER WEEK (INCHES):	0.106	0.139	0.228	0.318	0.416	0.489	0.579	0.497	0.391	0.253	0.122	0.073	
MINUTES OF WATER PER WEEK:	YEAR 1	1	2	3	4	5	6	7	6	5	3	2	1		
	YEAR 2	1	2	3	4	5	5	6	6	4	3	1	1		
DAYS PER WEEK:	YEAR 1	1	1	2	3	3	3	3	3	2	1	1	1		
	YEAR 2	1	1	2	3	3	3	3	3	2	1	1	1		
MINUTES OF WATER PER DAY:	YEAR 1	1	2	1	1	2	2	2	2	2	3	2	1		
	YEAR 2	1	2	1	1	2	2	2	2	2	3	1	1		
CYCLES PER DAY:	YEAR 1	1	1	1	1	1	1	1	1	1	1	1	1		
	YEAR 2	1	1	1	1	1	1	1	1	1	1	1	1		
MINUTES PER CYCLE:	YEAR 1	1	2	1	1	2	2	2	2	2	3	2	1		
	YEAR 2	1	2	1	1	2	2	2	2	2	3	1	1		

BUBBLER IRRIGATION FOR MODERATE WATER USE TREES – STATIONS: 29, 37, & 57															
SPRINKLER MANUFACTURER:		HUNTER		LOCATION:		NAPA, CALIFORNIA									
PRECIPITATION RATE (INCHES/HOUR):		1.50		HEAD SPACING:		VARIES									
IRRIGATION SYSTEM EFFICIENCY		0.85		HEAD GPM:		2 X .25									
PLANT FACTOR:		0.50													
YEAR 2 REDUCTION AMOUNT:		–10% OF YEAR 1 (ESTABLISHMENT) RUN TIME MINUTES													
MONTH:		JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC	TOTAL	
ETO PER MONTH (INCHES):		1.30	1.70	2.80	3.90	5.10	6.00	7.10	6.10	4.80	3.10	1.50	0.90	44.30	
ETO PER WEEK (INCHES):		0.300	0.393	0.647	0.901	1.178	1.386	1.640	1.409	1.109	0.716	0.346	0.208		
APPLIED ETO PER WEEK (INCHES):		0.177	0.231	0.380	0.530	0.693	0.815	0.965	0.829	0.652	0.421	0.204	0.122		
MINUTES OF WATER PER WEEK:	YEAR 1	7	9	15	21	28	33	39	33	26	17	8	5		
	YEAR 2	6	8	14	19	25	29	35	30	23	15	7	4		
DAYS PER WEEK:	YEAR 1	1	1	1	1	1	2	2	2	2	1	1	1		
	YEAR 2	1	1	1	1	1	2	2	2	2	1	1	1		
MINUTES OF WATER PER DAY:	YEAR 1	7	9	15	21	28	16	19	17	13	17	8	5		
	YEAR 2	6	8	14	19	25	15	17	15	12	15	7	4		
CYCLES PER DAY:	YEAR 1	1	1	1	1	1	1	1	1	1	1	1	1		
	YEAR 2	1	1	1	1	1	1	1	1	1	1	1	1		
MINUTES PER CYCLE:	YEAR 1	7	9	15	21	28	16	19	17	13	17	8	5		
	YEAR 2	6	8	14	19	25	15	17	15	12	15	7	4		

NOTES:
THE CHARTS ARE INTENDED TO BE USED AS A GUIDELINE ONLY AND INDICATE APPROXIMATE RUN TIMES (IN MINUTES) FOR EACH ZONE BASED ON ESTIMATED WEEKLY WATER REQUIREMENTS FOR ESTABLISHED PLANT MATERIAL. THE FIGURES SHOWN IN THIS SCHEDULE ARE APPROXIMATE AND HAVE BEEN DEVELOPED FROM LOCAL CURRENT AVERAGES FOR EVAPOTRANSPIRATION, AND REFLECT MAXIMUM IRRIGATION REQUIREMENTS OF THE PLANT MATERIAL BASED ON PLANT TYPE AND SPACING. ACTUAL RUN TIMES MAY BE DIFFERENT DEPENDING ON A VARIETY OF FACTORS INCLUDING TOPOGRAPHY, SOIL STRUCTURE, SUN AND WIND EXPOSURE, WEATHER, ACTUAL PLANT WATER REQUIREMENTS, ETC.

MP ROTATOR IRRIGATION FOR MODERATE WATER USE SHRUB/GROUNDCOVER – STATIONS: 12, 22, 23, 26, 27, 28, 30, 31, 32, 38, 39, 40, 41, 43, 45, 46, 47, 49, 50, 51, 52, 53, 54, 58, 59, 60, 62, 63, & 64															
SPRINKLER MANUFACTURER:		HUNTER		LOCATION:		NAPA, CALIFORNIA									
PRECIPITATION RATE (INCHES/HOUR):		0.45		HEAD SPACING:		VARIES									
IRRIGATION SYSTEM EFFICIENCY		0.75		HEAD GPM:		VARIES									
PLANT FACTOR:		0.50													
YEAR 2 REDUCTION AMOUNT:		–10% OF YEAR 1 (ESTABLISHMENT) RUN TIME MINUTES													
		MONTH:	JAN	FEB	MAR	APR	MAY	JUN	JUL	AUG	SEP	OCT	NOV	DEC	TOTAL
		ETO PER MONTH (INCHES):	1.30	1.70	2.80	3.90	5.10	6.00	7.10	6.10	4.80	3.10	1.50	0.90	44.30
		ETO PER WEEK (INCHES):	0.300	0.393	0.647	0.901	1.178	1.386	1.640	1.409	1.109	0.716	0.346	0.208	
		APPLIED ETO PER WEEK (INCHES):	0.200	0.262	0.431	0.600	0.785	0.924	1.093	0.939	0.739	0.477	0.231	0.139	
MINUTES OF WATER PER WEEK:	YEAR 1	27	35	57	80	105	123	146	125	99	64	31	18		
	YEAR 2	24	31	52	72	94	111	131	113	89	57	28	17		
DAYS PER WEEK:	YEAR 1	1	1	2	3	3	3	3	3	3	2	1	1		
	YEAR 2	1	1	2	3	3	3	3	3	3	2	1	1		
MINUTES OF WATER PER DAY:	YEAR 1	27	35	29	27	35	41	49	42	33	32	31	18		
	YEAR 2	24	31	26	24	31	37	44	38	30	29	28	17		
CYCLES PER DAY:	YEAR 1	1	1	1	1	1	1	1	1	1	1	1	1		
	YEAR 2	1	1	1	1	1	1	1	1	1	1	1	1		
MINUTES PER CYCLE:	YEAR 1	27	35	29	27	35	41	49	42	33	32	31	18		
	YEAR 2	24	31	26	24	31	37	44	38	30	29	28	17		

MP ROTATOR IRRIGATION FOR LOW WATER USE SHRUB/GROUNDCOVER – STATIONS: 1, 3, 10, 11, 13, 14, 20, 24, 25, & 66		
--	--	--


SECTION B: WATER EFFICIENT LANDSCAPE WORKSHEETS
Section B1. Hydrozone Information Table

Please complete the hydrozone table(s) for each hydrozone. Use as many tables as necessary to provide the square footage of landscape area per hydrozone.

Hydrozone*	Zone or Valve	Irrigation Method**	Area (Sq. Ft.)	% of Total Landscape Area
Moderate shrub/gc	moderate MP	rotor	37222	53.8%
Low shrub/gc	low MP	rotor	19294	27.9%
Moderate shrub/gc	moderate spray	spray	2389	3.5%
Low shrub/gc	low spray	spray	3148	4.5%
Low shrub/gc	low bubbler	bubbler	6104	8.8%
Moderate shrub/gc	moderate drip	drip	31	0.1%
Moderate trees	moderate bubbler	bubbler	1056	1.4%
Total (Sq. Ft.):			69,244	100%

Summary Hydrozone Table		
Hydrozone*	Area (Sq. Ft.)	% of Total Landscape Area
High Water Use	0	0
Moderate Water Use	40,698	59
Low Water Use	28,546	41
Total:	69,244	100%

*** Hydrozone**

HW = High Water Use Plants

MW = Moderate Water Use Plants

LW = Low Water Use Plants

****Irrigation Method**

MS = Micro-spray

S = Spray

R = Rotor

B = Bubbler

D = Drip



Section B2. Maximum Applied Water Allowance (MAWA)

The project's Maximum Applied Water Allowance shall be calculated using this equation:

$$\text{MAWA} = (\text{ETo}) (0.62) [(0.7 \times \text{LA}) + (0.3 \times \text{SLA})]$$

where:

MAWA = Maximum Applied Water Allowance (gallons per year)

ETo = Reference Evapotranspiration from *Appendix A* (inches per year)

0.7 = ET Adjustment Factor (ETAF)

LA = Landscaped Area includes Special Landscape Area (square feet)

0.62 = Conversion factor (to gallons per square foot)

SLA = Portion of the landscape area identified as Special Landscape Area (square feet)

0.3 = the additional ET Adjustment Factor for Special Landscape Area (1.0 - 0.7 = 0.3)

Maximum Applied Water Allowance = 1,331,299 gallons per year

Show calculations.

$$44.3 \times 0.62 \times (.7 \times 69,244) = 1,331,299$$

Effective Precipitation (Eppt)

If considering Effective Precipitation, use 25% (0.25) of annual precipitation. Use the following equation to calculate the Maximum Applied Water Allowance (see Appendix B for rainfall map):

$$\text{MAWA} = (\text{ETo} - \text{Eppt}) (0.62) [(0.7 \times \text{LA}) + (0.3 \times \text{SLA})]$$

Maximum Applied Water Allowance = _____ gallons per year

Show calculations.



Section B3. Estimated Total Water Use (ETWU)

The project's Estimated Total Water Use is calculated using the following formula:

$$ETWU = (ET_o) (0.62) \left(\frac{PF \times HA}{IE} + SLA \right)$$

where:

ETWU = Estimated total water use per year (gallons per year)

ET_o = Reference Evapotranspiration (inches per year)

PF = Plant Factor from WUCOLS² (*Water Use Classification of Landscape Species, UCCE 2000*)

HA = Hydrozone Area [high, medium, and low water use areas] (square feet)

SLA = Special Landscape Area (square feet)

0.62 = Conversion Factor (to gallons per square foot)

IE = Irrigation Efficiency (minimum 0.71)

Hydrozone Table for Calculating ETWU

Please complete the hydrozone table(s). Use as many tables as necessary.

Hydrozone	Plant Water Use Type(s)	Plant Factor (PF)	Area (HA) (square feet)	PF x HA (square feet)
Moderate MP	Moderate	.5	37,222	18,611
Low MP	Low	.3	19,294	5,788
Moderate Spray	Moderate	.5	2,389	1,195
Low Spray	Low	.3	3,148	944
Low Bubbler	Low	.3	6,104	1,831
Moderate Drip	Moderate	.5	31	16
Moderate Bubbler	Moderate	.5	1,056	528

Estimated Total Water Use = 1,037,134 gallons

Show calculations.

1. 44.3 x .62 x 18611/.75 = 681,560	2. 44.3 x .62 x 5788/.75 = 211,964
3. 44.3 x .62 x 1195/.7 = 46,888	4. 44.3 x .62 x 944/.7 = 37,040
5. 44.3 x .62 x 1831/.85 = 59,165	6. 44.3 x .62 x 16/.85 = 517
TOTAL = 1,037,134	

² To obtain plant factors from WUCOLS, see <http://www.water.ca.gov/wateruseefficiency/docs/wucols00.pdf> - *Water Use Classification of Landscape Species, UCCE 2000*.

80 Clear Creek Road
Water Availability Analysis
July 11, 2025

SUMMIT ENGINEERING, INC.
Project No.: 2021275

ENCLOSURE D

EXISTING LANDSCAPE AND IRRIGATION PLANS FOR APN 027-310-043

SUMMIT ENGINEERING, INC.	80 Clear Creek Road Water Use Estimates APN 027-310-043 Landscape Irrigation Demand	PROJECT NO. 2021275 BY: TBL CHK: GG
--------------------------	--	--

Reference ET 46.6
(Value listed in California WELO Appendix C for Oakville, CA)

Plant factors
Tree plant factor 0.5
(Recommended value for all trees by University of California Agriculture and Natural Resources:
<https://ucanr.edu/site/center-landscape-urban-horticulture/estimating-tree-water-requirements>)

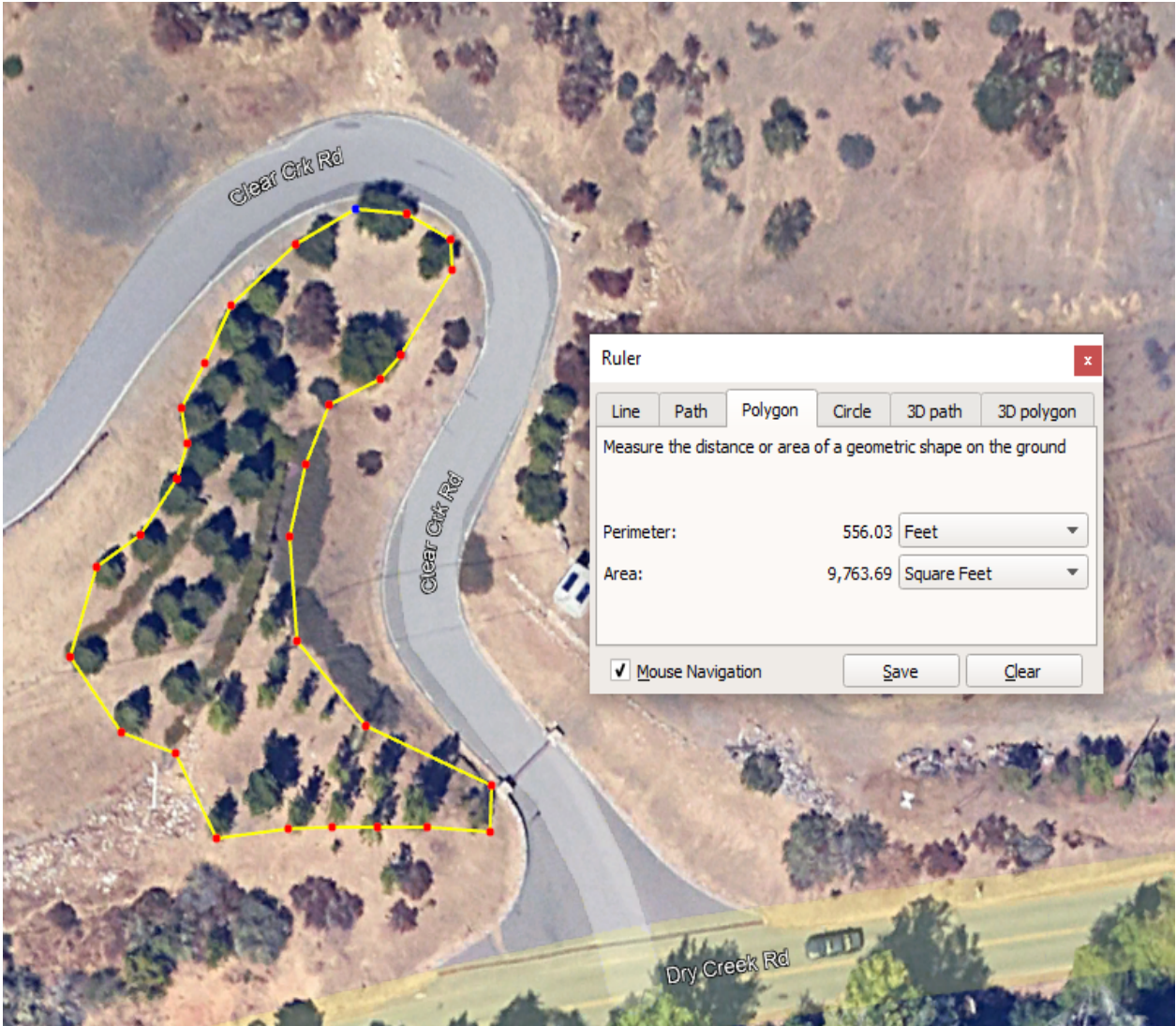
Irrigation efficiency 0.81
(WELO-recommended value for drip irrigation)

Canopy cover areas
Area 1 9,764 sf
Area 2 464 sf
Total Area 10,228 sf
(Canopy cover areas conservatively estimated as shown on attached aerial image)

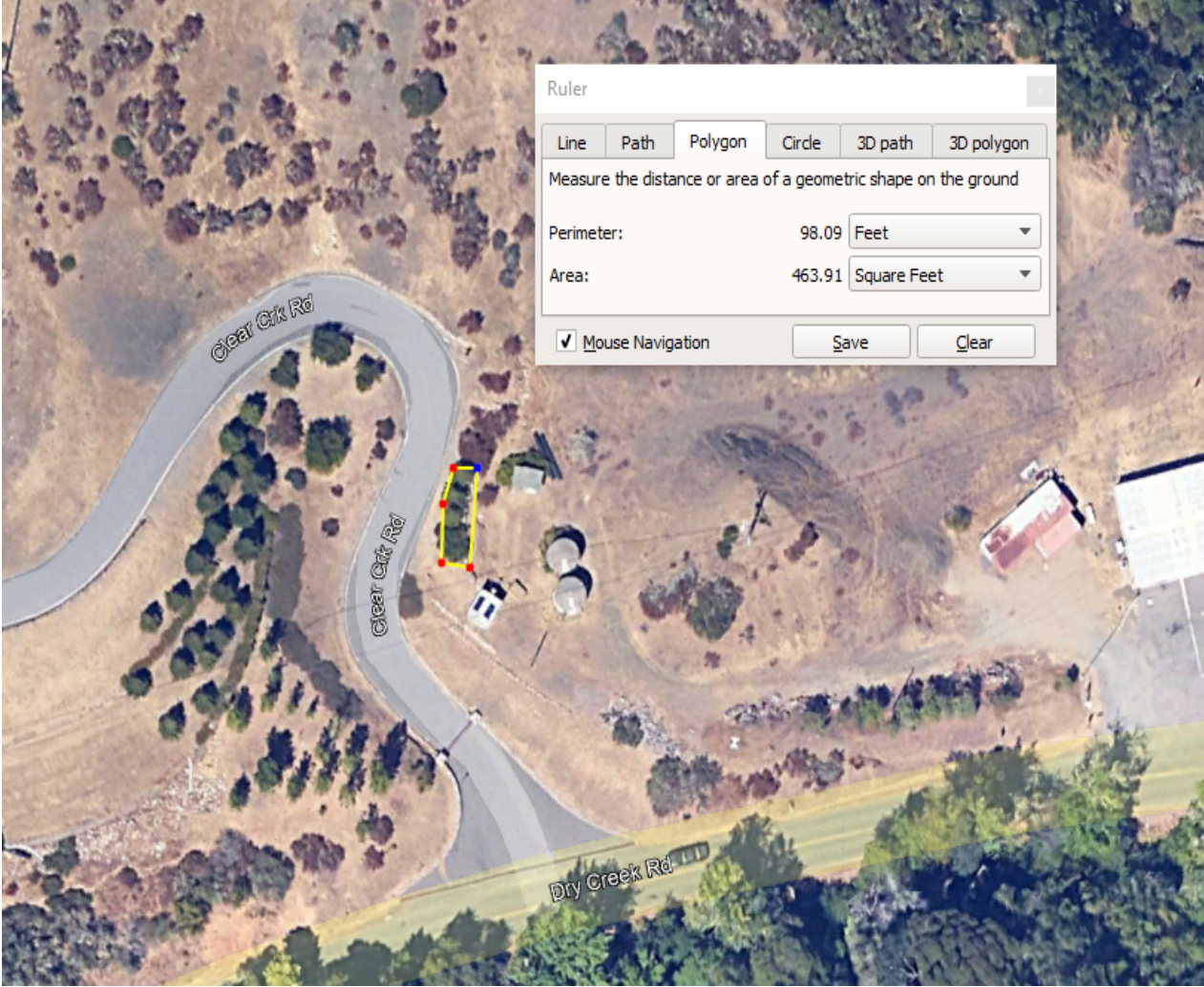
Calculated water demand
(Based on WELO ETWU methodology)
174,137 gal/yr
8,275 gal/yr

Total:	182,412 gal/yr
=	0.56 af/yr

Area 1



Area 2

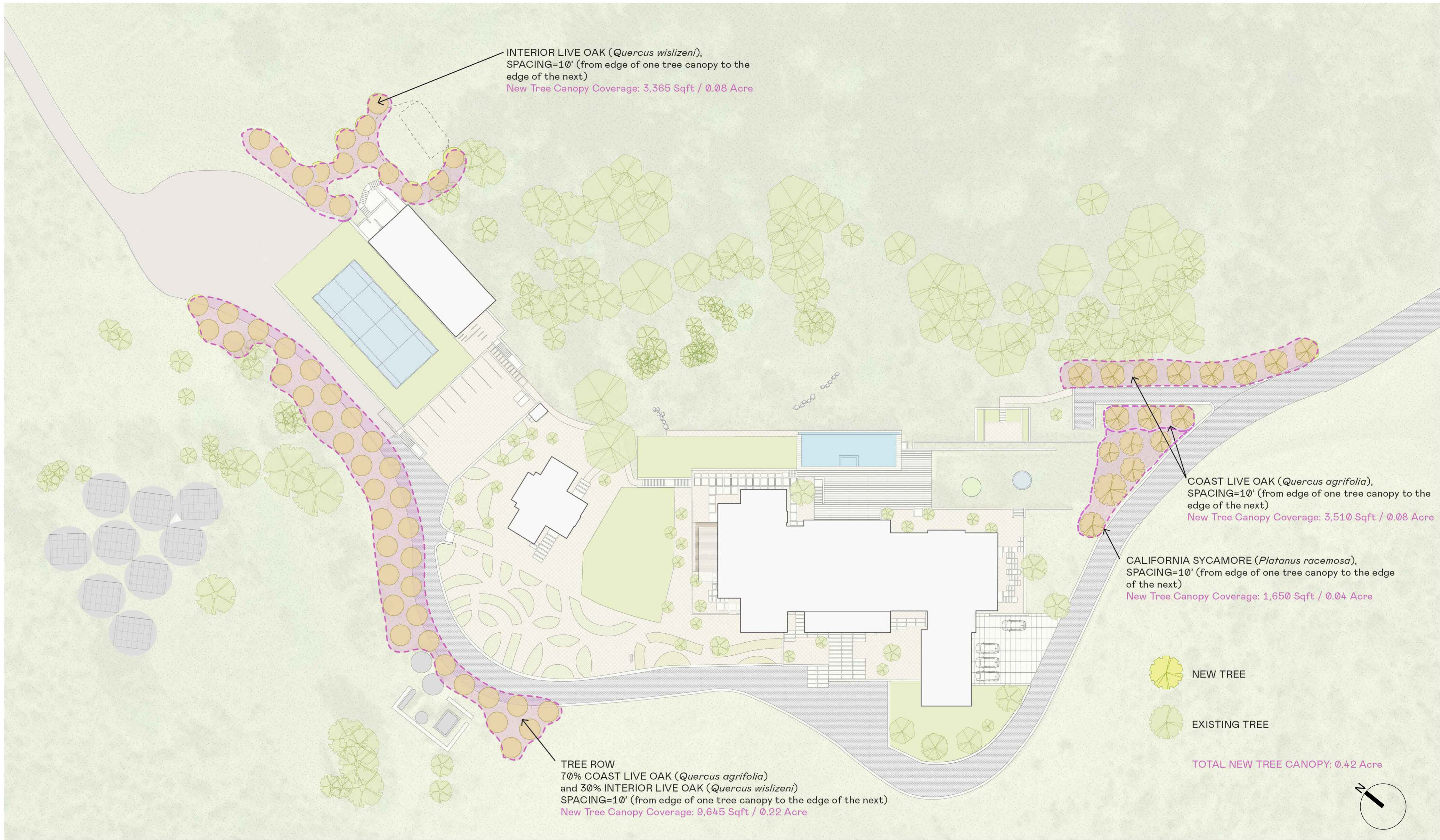


80 Clear Creek Road
Water Availability Analysis
July 11, 2025

SUMMIT ENGINEERING, INC.
Project No.: 2021275

ENCLOSURE E

PROPOSED LANDSCAPE PLANS



INTERIOR LIVE OAK (*Quercus wislizeni*),
SPACING=10' (from edge of one tree canopy to the
edge of the next)
New Tree Canopy Coverage: 3,365 Sqft / 0.08 Acre

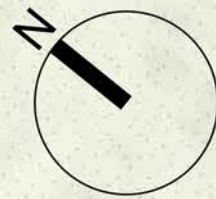
COAST LIVE OAK (*Quercus agrifolia*),
SPACING=10' (from edge of one tree canopy to the
edge of the next)
New Tree Canopy Coverage: 3,510 Sqft / 0.08 Acre

CALIFORNIA SYCAMORE (*Platanus racemosa*),
SPACING=10' (from edge of one tree canopy to the edge
of the next)
New Tree Canopy Coverage: 1,650 Sqft / 0.04 Acre

TREE ROW
70% COAST LIVE OAK (*Quercus agrifolia*)
and 30% INTERIOR LIVE OAK (*Quercus wislizeni*)
SPACING=10' (from edge of one tree canopy to the edge of the next)
New Tree Canopy Coverage: 9,645 Sqft / 0.22 Acre



TOTAL NEW TREE CANOPY: 0.42 Acre



TERRE
MOTO

300 BRANNAN ST
SAN FRANCISCO, CA 94107
WWW.TERREMOTO.LA



PROJECT

PROJECT CLOUD

Ponderosa One, LLC
80 Clear Creek Rd., Oakville
CA 94562

ISSUE SET

SCHEMATIC DESIGN

ISSUE DATE

12/06/2024

REV DATE

REV	DATE	ISSUE	BY
1	04.15.24	Viewshed Revision	YS
2	12.06.24	New Tree Coverage	YS

DRAWN BY:

CHECKED BY:

SCALE:

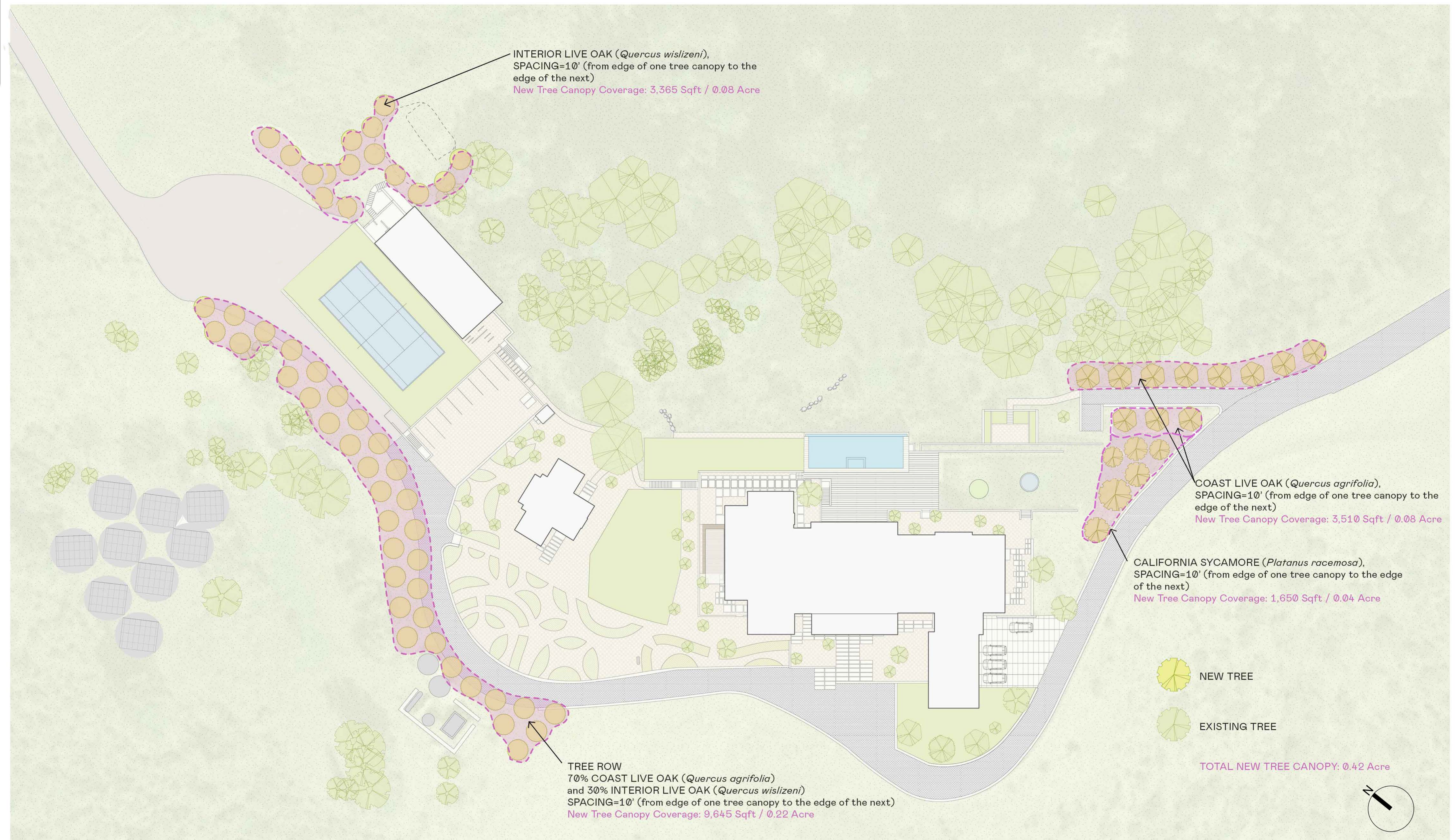
1"=30'-0"

SHEET TITLE

NEW TREES

PROJECT NUMBER

LA2



INTERIOR LIVE OAK (*Quercus wislizeni*),
SPACING=10' (from edge of one tree canopy to the
edge of the next)
New Tree Canopy Coverage: 3,365 Sqft / 0.08 Acre

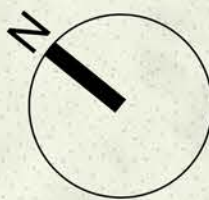
COAST LIVE OAK (*Quercus agrifolia*),
SPACING=10' (from edge of one tree canopy to the
edge of the next)
New Tree Canopy Coverage: 3,510 Sqft / 0.08 Acre

CALIFORNIA SYCAMORE (*Platanus racemosa*),
SPACING=10' (from edge of one tree canopy to the edge
of the next)
New Tree Canopy Coverage: 1,650 Sqft / 0.04 Acre

TREE ROW
70% COAST LIVE OAK (*Quercus agrifolia*)
and 30% INTERIOR LIVE OAK (*Quercus wislizeni*)
SPACING=10' (from edge of one tree canopy to the edge of the next)
New Tree Canopy Coverage: 9,645 Sqft / 0.22 Acre

- NEW TREE
- EXISTING TREE

TOTAL NEW TREE CANOPY: 0.42 Acre



TERRE MOTO

300 BRANNAN ST
SAN FRANCISCO, CA 94107
WWW.TERREMOTO.LA



PROJECT

PROJECT CLOUD

Ponderosa One, LLC
80 Clear Creek Rd., Oakville
CA 94562

ISSUE SET

SCHEMATIC DESIGN

ISSUE DATE

12/06/2024

REV DATE

ISSUE

BY

1

04.15.24

Viewshed Revision

YS

2

12.06.24

New Tree Coverage

YS

DRAWN BY:

CHECKED BY:

SCALE

1"=30'-0"

SHEET TITLE

NEW TREES



TERRE
MOTO

3486-A MISSION ST
SAN FRANCISCO, CA 94110
843-271-7278
WWW.TERREMOTO.LA



PROJECT

PROJECT CLOUD
Ponderosa One, LLC
80 Clear Creek Rd., Oakville
CA 94562

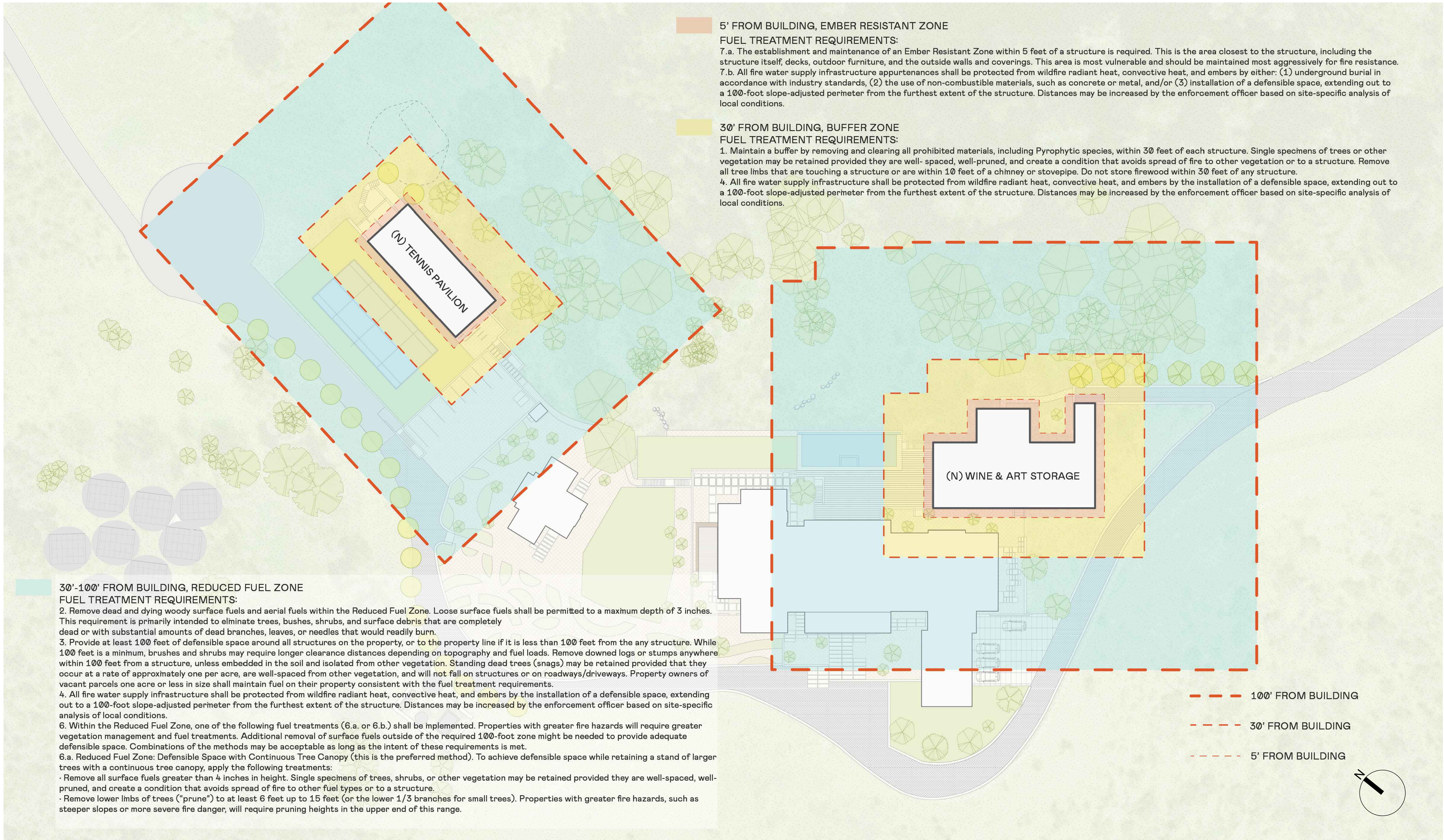
ISSUE SET	SCHEMATIC DESIGN		
ISSUE DATE	04/15/2024		
REV	DATE	ISSUE	BY
1	04-15-24	Viewshed Revision	YS

DRAWN BY:
CHECKED BY:

SCALE
1"=30'-0"

SHEET TITLE
LIGHTING PLAN

SHEET NUMBER
LA3



TERRE
MOTO

3486-A MISSION ST
SAN FRANCISCO, CA 94110
843-271-7278
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PROJECT

PROJECT CLOUD

Ponderosa One, LLC
80 Clear Creek Rd., Oakville
CA 94562

ISSUE SET

SCHEMATIC DESIGN

ISSUE DATE

04/15/2024

REV

DATE

ISSUE

BY

1

04-15-24

Viewshed Revision

YS

DRAWN BY:

CHECKED BY:

SCALE

1"=30'-0"

SHEET TITLE

DEFENSIBLE SPACE
NEW BUILDING

SHEET NUMBER

LA4

80 Clear Creek Road
Water Availability Analysis
July 11, 2025

SUMMIT ENGINEERING, INC.
Project No.: 2021275

ENCLOSURE F

EXCERPT FROM LUDHORFF & SCALMANINI REPORT WITH RECHARGE DATA

around the MST Subarea to determine the primary mechanisms of groundwater recharge. A seepage experiment consists of several streamflow measurements taken along the length of a stream to quantify streamflow gains and losses. The stream is considered losing where streamflow decreases between measurements, and gaining where streamflow increases. He concluded that the infiltration rate from precipitation and runoff is greatest where tuffs are exposed or underlie shallow Quaternary deposits. Additionally, only a small percentage of groundwater recharge was found to come from direct precipitation, but instead it is greatest where streams and tributaries come in contact with tuffs. Farrar and Metzger (2003) similarly analyzed seepage gains/losses for various creeks and tributaries in the MST. They concluded that significant streambed infiltration also occurs where streamflow passes over unconsolidated, highly permeable, alluvial deposits. **Figure 6-1** is a conceptual illustration of the major surface and subsurface hydrologic processes occurring within a watershed and shows how the hydrogeology of the Napa Valley area relates to these processes. As illustrated in **Figure 6-1** and discussed in greater detail in Sections 7 and 8 of this report, precipitation falling within the watershed infiltrates the ground or becomes surface water outflow through surface runoff processes. Some fraction of infiltrated water is consumed through plant evapotranspiration and some water percolates deeper and into the aquifer system as recharge. The potential for water to recharge the groundwater system depends on many factors, including the nature of the geologic materials and topography.

Based on the findings of Johnson (1977) and Farrar and Metzger (2003), a map was created to locate areas of greatest recharge potential. This map shows the location of exposed tuffs throughout the county (**Figure 6-2**). Sonoma Volcanics sedimentary deposits and various alluvial units found countywide were also included in the map following findings by Farrar and Metzger (2003). Areas in which the slope of the land surface exceeds 30 degrees, beyond which recharge potential is significantly reduced, were also added to the map.

Two sizeable exposures of rhyolitic ash-flow tuff and related alluvium occur in the northern portion of the Eastern and Western Mountains near Calistoga. The eastern exposure covers roughly 30 square miles with tuff in the north and Sonoma Volcanics sedimentary deposits to the south. Following Johnson (1977), the greatest recharge would be expected along Bell Creek, which traverses much of the northern tuffs, and Conn Creek, which passes over large Sonoma Volcanic sedimentary deposits in Conn Valley, some of which are covered by younger alluvium. The Western Mountains exposure, which covers roughly 18 square miles, is almost entirely tuff, with a single Sonoma Volcanics sedimentary deposit in the north at Cyrus Creek. Again, following Johnson (1977), the greatest recharge potential would be expected along York, Mill, Richie, Nash, and Cyrus Creeks (**Figure 6-2**). Although concealed below the Napa Valley Floor, it is likely that the two exposures are connected at depth. It is expected that much of the water recharged through these two exposures eventually reaches the aquifer units of the Napa Valley Floor and flows to the south.

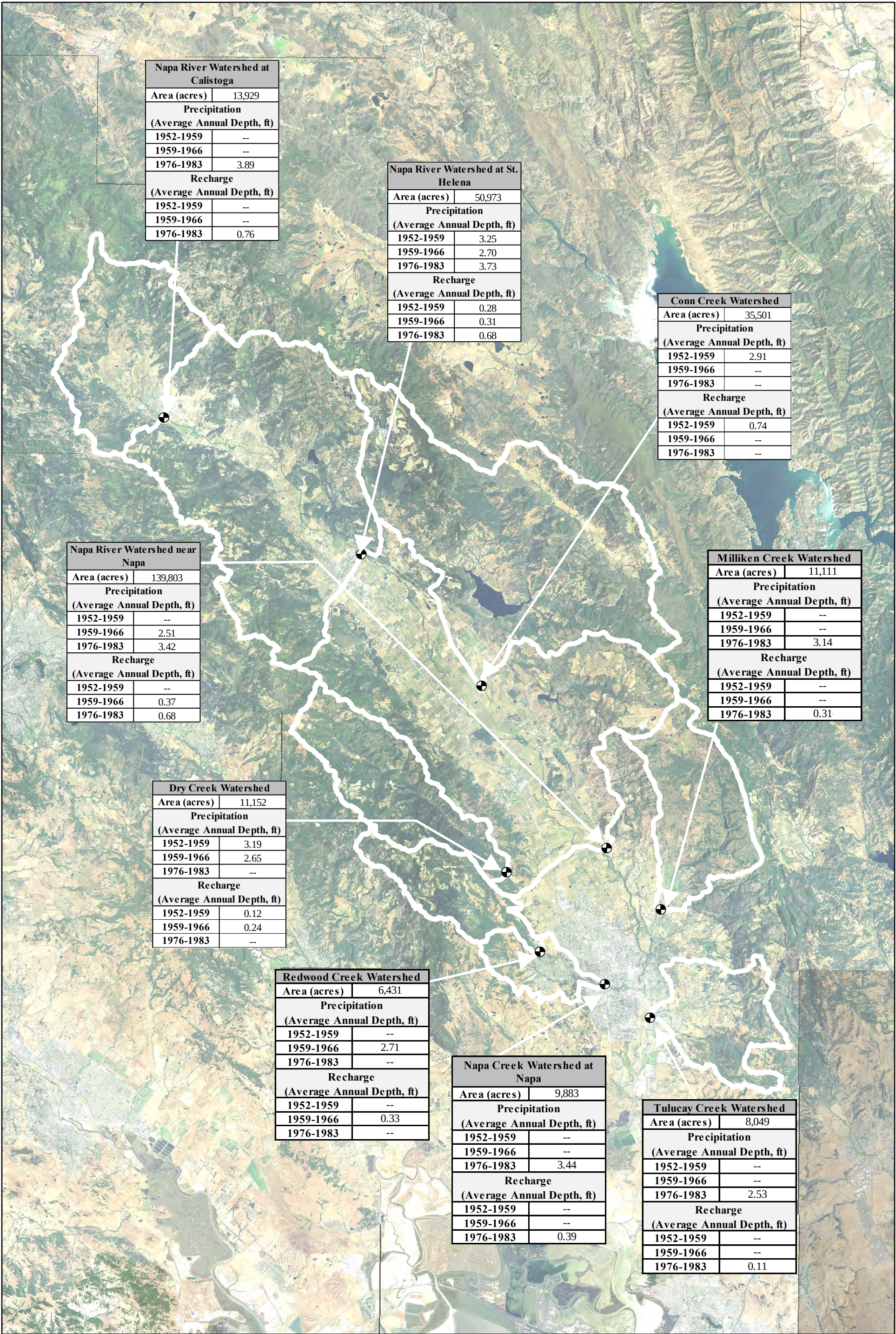
Table 8-9. Summary of Water Balance Model Results

	Average Annual (acre-feet)					Range (acre-feet)	Recharge (% of Precip.)
Watershed	Precip.	Outflow	Infilt.	ET	Recharge	Recharge	Recharge
Napa River near Napa	418,500	146,800	271,700	201,900	70,600	8,300 - 185,900	17%
- Conn Creek	98,200	24,600	73,600	52,200	21,100	4,300 - 40,700	21%
- Dry Creek	33,000	14,200	18,700	16,400	2,000	500 - 6,300	6%
- Napa River at St. Helena	161,400	67,000	94,400	72,500	22,000	2,500 - 60,900	14%
- Napa River at Calistoga	54,200	23,600	30,600	19,700	10,500	2,000 - 17,200	19%
Milliken Creek	33,000	16,800	16,200	13,500	2,500	100 - 7,100	8%
Tulucay Creek	19,500	9,100	10,400	9,500	1,000	100 - 2,300	5%
Redwood Creek	19,300	7,800	11,500	9,500	1,900	400 - 5,000	10%
Napa Creek at Napa	32,100	14,800	17,300	13,700	3,600	600 - 6,900	11%

Results presented in **Table 8-9** indicate that within the Napa River near Napa watershed groundwater recharge is higher in the Conn Creek watershed and in the northern portion of the watershed above Calistoga. Less recharge occurs in the Dry Creek watershed and the portion of the watershed between Calistoga and St. Helena. The Tulucay Creek watershed has the lowest estimated groundwater recharge equal to only 5% of precipitation.

A method for comparing absolute groundwater recharge between watersheds involves comparing groundwater recharge results as depth (normalized by area) for common hydrologic periods. Groundwater recharge can be expressed as depth by dividing average annual recharge volume by the watershed area. To facilitate such a comparison, three common hydrologic periods of eight years each were selected for comparisons of at least two different watersheds for each period. Common periods of record included water years 1952 through 1959, 1959 through 1966, and 1976 through 1983. Average annual groundwater recharge depths were calculated for each watershed during these periods. Average annual precipitation as depth over the watershed was also calculated to provide an indication of the relative wetness of the three common periods. These results are presented in **Figure 8-13**. The Conn Creek watershed was considered in this analysis for the period after construction of Lake Hennessey.

Results presented in **Figure 8-13** illustrate similar trends as seen in **Table 8-9**. The period from 1959 through 1966 was the driest of the three periods while the 1976 through 1983 period was the wettest. Based on absolute groundwater recharge depth, recharge was generally highest in



Napa River Watershed at Calistoga	
Area (acres)	13,929
Precipitation (Average Annual Depth, ft)	
1952-1959	--
1959-1966	--
1976-1983	3.89
Recharge (Average Annual Depth, ft)	
1952-1959	--
1959-1966	--
1976-1983	0.76

Napa River Watershed at St. Helena	
Area (acres)	50,973
Precipitation (Average Annual Depth, ft)	
1952-1959	3.25
1959-1966	2.70
1976-1983	3.73
Recharge (Average Annual Depth, ft)	
1952-1959	0.28
1959-1966	0.31
1976-1983	0.68

Conn Creek Watershed	
Area (acres)	35,501
Precipitation (Average Annual Depth, ft)	
1952-1959	2.91
1959-1966	--
1976-1983	--
Recharge (Average Annual Depth, ft)	
1952-1959	0.74
1959-1966	--
1976-1983	--

Napa River Watershed near Napa	
Area (acres)	139,803
Precipitation (Average Annual Depth, ft)	
1952-1959	--
1959-1966	2.51
1976-1983	3.42
Recharge (Average Annual Depth, ft)	
1952-1959	--
1959-1966	0.37
1976-1983	0.68

Milliken Creek Watershed	
Area (acres)	11,111
Precipitation (Average Annual Depth, ft)	
1952-1959	--
1959-1966	--
1976-1983	3.14
Recharge (Average Annual Depth, ft)	
1952-1959	--
1959-1966	--
1976-1983	0.31

Dry Creek Watershed	
Area (acres)	11,152
Precipitation (Average Annual Depth, ft)	
1952-1959	3.19
1959-1966	2.65
1976-1983	--
Recharge (Average Annual Depth, ft)	
1952-1959	0.12
1959-1966	0.24
1976-1983	--

Redwood Creek Watershed	
Area (acres)	6,431
Precipitation (Average Annual Depth, ft)	
1952-1959	--
1959-1966	2.71
1976-1983	--
Recharge (Average Annual Depth, ft)	
1952-1959	--
1959-1966	0.33
1976-1983	--

Napa Creek Watershed at Napa	
Area (acres)	9,883
Precipitation (Average Annual Depth, ft)	
1952-1959	--
1959-1966	--
1976-1983	3.44
Recharge (Average Annual Depth, ft)	
1952-1959	--
1959-1966	--
1976-1983	0.39

Tulucay Creek Watershed	
Area (acres)	8,049
Precipitation (Average Annual Depth, ft)	
1952-1959	--
1959-1966	--
1976-1983	2.53
Recharge (Average Annual Depth, ft)	
1952-1959	--
1959-1966	--
1976-1983	0.11



1771 Tribute Road, Suite A
Sacramento, CA 95815
Phone: 916-456-4400 - Fax: 916-456-0253

Figure 8-13: Napa Valley Area Recharge by Hydrologic Period

SCALE:	1" = 3.0 miles
JOB NUMBER:	3327
DRAWN BY:	P.Ho
DATE:	11-26-2012

80 Clear Creek Road
Water Availability Analysis
July 11, 2025

SUMMIT ENGINEERING, INC.
Project No.: 2021275

ENCLOSURE G

PRISM RAINFALL DATA FROM 2015–2024

PRISM Time Series Data

Location: Lat: 38.4156 Lon: -122.4412 Elev: 1093ft

Climate variable: ppt

Spatial resolution: 800m

Period: 2015 - 2024

Dataset: AN91m

PRISM day definition: 24 hours ending at 1200 UTC on the day shown

Grid Cell Interpolation: Off

Time series generated: 2025-Jul-07

Details: http://www.prism.oregonstate.edu/documents/PRISM_datasets.pdf

Date	ppt (inches)
------	--------------

2015	17.24
------	-------

2016	46.87
------	-------

2017	52.13
------	-------

2018	29.97
------	-------

2019	55.3
------	------

2020	12.57
------	-------

2021	35.48
------	-------

2022	20.98
------	-------

2023	48.53
------	-------

2024	52.98
------	-------

Average	37.21
---------	-------

80 Clear Creek Road
Water Availability Analysis
July 11, 2025

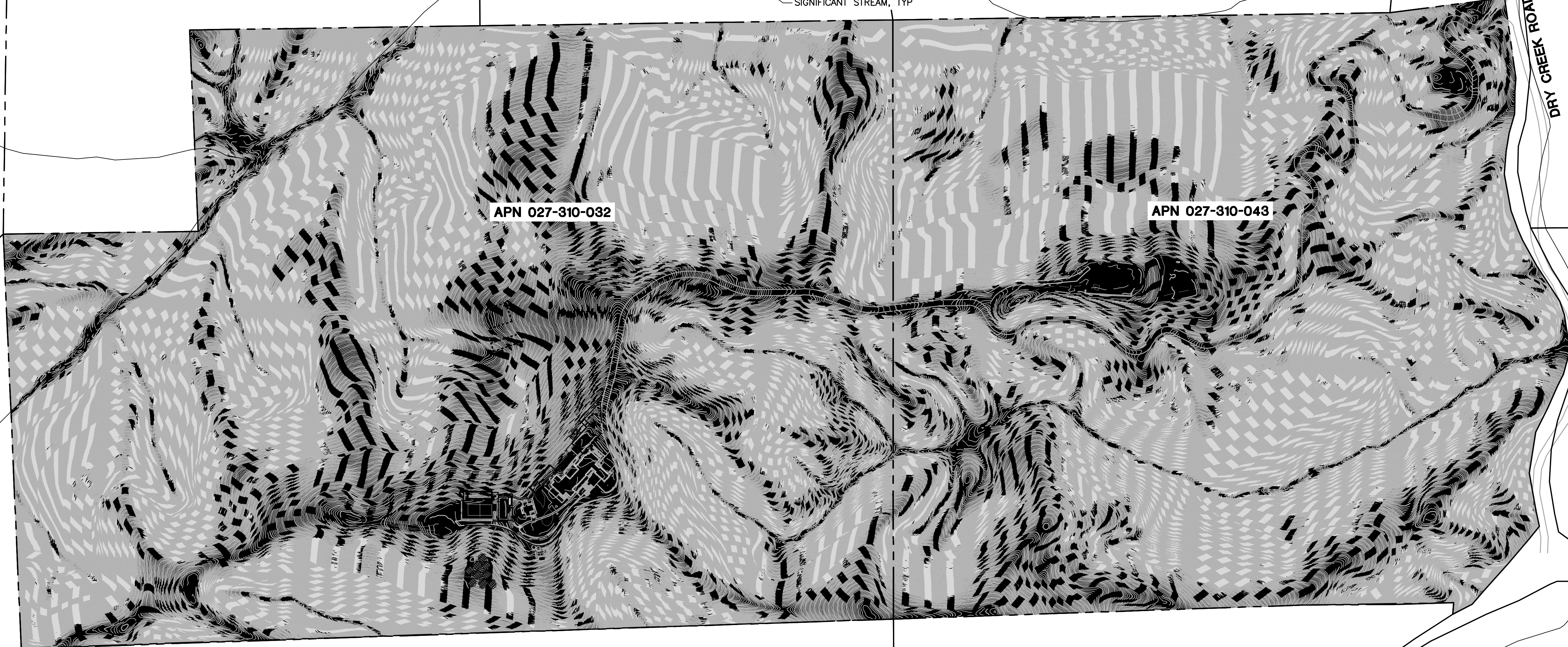
SUMMIT ENGINEERING, INC.
Project No.: 2021275

ENCLOSURE H

SITE SLOPE ANALYSIS

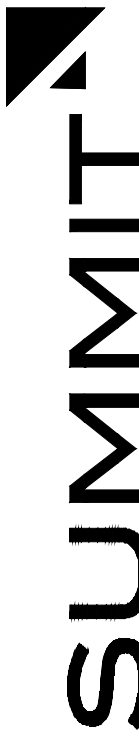
P:\2021\2021275 80 CLEAR CREEK ROAD\CAD\WWW\WAA SLOPE ANALYSIS\21275-VIEWSHD OSP.DWG

PLOTTED ON: 7/10/2025 7:06 PM



SLOPE ANALYSIS - APN 027-310-032				
Number	Minimum Slope	Maximum Slope	Color	Area (sf)
1	0.00%	57.00%		2268812.87
2	57.00%	638.91%		3091457.46

SLOPE ANALYSIS - APN 027-310-043				
Number	Minimum Slope	Maximum Slope	Color	Area (sf)
1	0.00%	57.00%		1616780.70
2	57.00%	157.78%		2441358.51



SUMMIT
Summit Engineering, Inc
575 W College Ave., Suite 201 • Santa Rosa, CA 95401
707-527-0775 • www.summit-sr.com

PONDEROSA ONE LLC
80 CLEAR CREEK ROAD
NAPA, CA 94558
APN 027-310-032 & 043

80 CLEAR CREEK WAA

SLOPE ANALYSIS

#	DATE	DESCRIPTION
	7/11/2025	WAA SUBMITAL

JOB NO: 2021275
SCALE: AS SHOWN
DRAWN: TF/TBL
CHECKED: GG
SHEET

**OSP**
SHEET 1 OF 1

80 Clear Creek Road
Water Availability Analysis
July 11, 2025

SUMMIT ENGINEERING, INC.
Project No.: 2021275

ENCLOSURE I

TIER 2 WELL DRAWDOWN CALCULATIONS

SUMMIT ENGINEERING, INC.	80 Clear Creek Rd Water Availability Tier II: Well Drawdown Analysis Well A	PROJECT NO. 2021275 BY: TBL CHK: GG
---------------------------------	--	--

Site Specific Parameters

Well Flow:	Low End Specific Storage:
25 gpm	1.50E-05 1/ft
Radius of Influence:	High End Specific Storage:
200 ft	3.10E-04 1/ft
Aquifer Thickness	Low Hydraulic Conductivity:
140 ft	10 ft/day
Pumping Time:	High Hydraulic Conductivity:
1 day	30 ft/day

Theis Drawdown

Scenario	Specific Storage (1/ft):	Hydraulic Conductivity (ft/day)	Theis u value (unitless):	u_a , rounded down (unitless):	u_b , rounded up (unitless):	$W(u_a)$	$W(u_b)$	$W(u)$, interpolated	Theis s value	Drawdown (ft)
High S, Low h	3.10E-04	10	2.21E-03	2.00E-03	3.00E-03	5.639	5.235	5.55	0.0079	1.52
Low S, Low h	1.50E-05	10	1.07E-04	1.00E-04	2.00E-04	8.633	7.94	8.58	0.0122	2.35
High S, High h	3.10E-04	30	7.38E-04	7.00E-04	8.00E-04	6.688	6.555	6.64	0.0031	0.61
Low S, High h	1.50E-05	30	3.57E-05	3.00E-05	4.00E-05	9.837	9.55	9.67	0.0046	0.88

Notes:

- 1) Retrieve hydraulic conductivity from Napa WAA map; Specific Storage from well drilling lithology/soil type
- 2) 4 conditions (varying specific storage and hydraulic conductivity) are considered
- 3) Low specific storage and low hydraulic conductivity typically will result in max drawdown (highlighted in green)
- 4) Drawdowns greater than 10 ft for 6-inch casings and 15 ft for 8-inch casings typically indicate significant impacts
- 5) Min and max Specific storage and conductivity values can be adjusted to be site specific

SUMMIT ENGINEERING, INC.	80 Clear Creek Rd Water Availability Tier II: Well Drawdown Analysis Well B	PROJECT NO. 2021275 BY: TBL CHK: GG
---------------------------------	--	--

Site Specific Parameters

Well Flow:	Low End Specific Storage:
16 gpm	1.50E-05 1/ft
Radius of Influence:	High End Specific Storage:
470 ft	3.10E-04 1/ft
Aquifer Thickness	Low Hydraulic Conductivity:
120 ft	10 ft/day
Pumping Time:	High Hydraulic Conductivity:
1 day	30 ft/day

Theis Drawdown

Scenario	Specific Storage (1/ft):	Hydraulic Conductivity (ft/day)	Theis u value (unitless):	u_a , rounded down (unitless):	u_b , rounded up (unitless):	$W(u_a)$	$W(u_b)$	$W(u)$, interpolated	Theis s value	Drawdown (ft)
High S, Low h	3.10E-04	10	1.43E-02	1.00E-02	2.00E-02	4.038	3.355	3.75	0.0040	0.77
Low S, Low h	1.50E-05	10	6.90E-04	6.00E-04	7.00E-04	6.842	6.688	6.70	0.0071	1.37
High S, High h	3.10E-04	30	4.76E-03	4.00E-03	5.00E-03	4.948	4.726	4.78	0.0017	0.33
Low S, High h	1.50E-05	30	2.30E-04	2.00E-04	3.00E-04	7.94	7.535	7.82	0.0028	0.53

Notes:

- 1) Retrieve hydraulic conductivity from Napa WAA map; Specific Storage from well drilling lithology/soil type
- 2) 4 conditions (varying specific storage and hydraulic conductivity) are considered
- 3) Low specific storage and low hydraulic conductivity typically will result in max drawdown (highlighted in green)
- 4) Drawdowns greater than 10 ft for 6-inch casings and 15 ft for 8-inch casings typically indicate significant impacts
- 5) Min and max Specific storage and conductivity values can be adjusted to be site specific

SUMMIT ENGINEERING, INC.	80 Clear Creek Rd Water Availability Tier II: Well Drawdown Analysis Well D	PROJECT NO. 2021275 BY: TBL CHK: GG
---------------------------------	--	--

Site Specific Parameters

Well Flow:	Low End Specific Storage:
16 gpm	1.50E-05 1/ft
Radius of Influence:	High End Specific Storage:
35 ft	3.10E-04 1/ft
Aquifer Thickness	Low Hydraulic Conductivity:
100 ft	10 ft/day
Pumping Time:	High Hydraulic Conductivity:
1 day	30 ft/day

Theis Drawdown

Scenario	Specific Storage (1/ft):	Hydraulic Conductivity (ft/day)	Theis u value (unitless):	u_a , rounded down (unitless):	u_b , rounded up (unitless):	$W(u_a)$	$W(u_b)$	$W(u)$, interpolated	Theis s value	Drawdown (ft)
High S, Low h	3.10E-04	10	9.49E-05	9.00E-05	1.00E-04	8.739	8.633	8.69	0.0111	2.13
Low S, Low h	1.50E-05	10	4.59E-06	4.00E-06	5.00E-06	11.85	11.63	11.72	0.0149	2.87
High S, High h	3.10E-04	30	3.16E-05	3.00E-05	4.00E-05	9.837	9.55	9.79	0.0042	0.80
Low S, High h	1.50E-05	30	1.53E-06	1.00E-06	2.00E-06	13.24	12.55	12.87	0.0055	1.05

Notes:

- 1) Retrieve hydraulic conductivity from Napa WAA map; Specific Storage from well drilling lithology/soil type
- 2) 4 conditions (varying specific storage and hydraulic conductivity) are considered
- 3) Low specific storage and low hydraulic conductivity typically will result in max drawdown (highlighted in green)
- 4) Drawdowns greater than 10 ft for 6-inch casings and 15 ft for 8-inch casings typically indicate significant impacts
- 5) Min and max Specific storage and conductivity values can be adjusted to be site specific

80 Clear Creek Road
Water Availability Analysis
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Project No.: 2021275

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